Concrete Distress – Assessments and Solutions

2017 Municipal Streets Seminar November 15, 2017 Ames, Iowa



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Distress Guide 2017

National Concrete Pavement Technology Center

DRAFT



GUIDE FOR

CONCRETE PAVEMENT DISTRESS ASSESSMENTS AND SOLUTIONS

IDENTIFICATION, COMPREHENSION, PREVENTION AND REPAIR



- Surface Defects
- Surface Delamination
- Material Related Cracks
- Transverse & Diagonal Cracking
- Longitudinal Cracking
- Corner Cracking
- Spalling
- Faulting
- Joint Warping and Curling
- Blowups
- Settlement and Heaves
- Subgrades & Base Support Conditions
- CRCP
- Concrete Overlays, BCOA, BCOC, UBCOA, UBCOC
- Laboratory & Field Testing

Focus:

- Identification
- Causes
- Prevention
- Rehabilitation

Surface Defects



Map Crazing



Scaling

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Plastic Shrinkage Cracking



Popouts



Summary of Causes & Prevention of Surface Defects

Г	revention	i Oi Suita	ace Dele	ECIS
			Prevention of	or Mitigation
Distress	Causes	Design	Material Selection	Construction
I. Map Cracking (Crazing)	 Overworking/over -finishing of concrete surface Finishing while bleed water is present on surface Late or inadequate curing Batching absorptive aggregates that are on the dry side of SSD 	 Design concrete with low permeability Use moderate slump mixtures (with low w/cm ratio) 	 Use blended cements or SCMs to control AAR Use durable, nonreactive aggregates. 	 Do not: Overwork surface Spray water on surface during finishing Finish while bleed water is present. Sprinkle dry cement on surface to dry bleed water Begin curing as soon as possible. Keep stockpiles wet when absorptive aggregates used
2. Plastic Shrinkage Cracking	 Rapid evaporation of moisture from the concrete surface 	Use durable mixtures with low w/cmMinimize	 Reduce aggregate absorption of mix water 	 Employ proper hot-weather and cold-weather paving practices, as appropriate Begin curing as soon as

cement

possible

Summary of Causes & Prevention of Surface Defects

Prevention of Surface Defects				
		Prevention or Mitigation		
Distress	Causes	Design	Material Selection	Construction
3. Scaling	 Excessive use of deicing salts and freeze-thaw cycles Use of poor finishing and curing practices Failure to protect surface of newly placed fresh concrete from rain 	 Ensure proper air-void system parameters (adequate air content and spacing factor) in concrete Use appropriate amounts of SCMs 		 Ensure effective curing Do not: Overwork surface Spray water on surface during finishing Finish while bleed water is present Sprinkle dry cement on surface to dry bleed water Protect slab from rain
4. Surface Polishing or Surface Wear	 Use of soft aggregates with poor abrasion resistance Use of poor surface finishing 	 Use concrete mixtures with adequate strength 	 Use hard wear- resistant aggregate 	 Use proper finishing practices. Employ effective curing practices

and curing practices.

Summary of Causes & Prevention of Surface Defects

		Prevention or Mitigation		
Distress	Causes	Design	Material Selection	Construction
5. Popouts or Mortar Flaking	 Use of unsound or reactive aggregates Aggregate expansion upon freezing (popout) Excessive Vibration of concrete 	Use durable mixtures with low w/cm	 Use only sound, nonreactive aggregates that have been tested for undesirable fine particles 	 Begin curing as soon as possible. Use evaporation retarders to minimize moisture loss Use effective stockpile management practices to minimize contaminants (see ACPA 2004)



Treatment & Repairs

- Penetrating Sealers
 - for surface defects caused by deicing salts
- High Molecular Weight Methacrylate (HMWM)
 - achieves excellent penetration into cracks and can serve to strengthen the concrete by filling the crack and bonding it together
- Void Filling
 - cementitious, epoxy, and proprietary materials have all been used successfully to fill and repair clay ball voids
- Slab Replacement
- Tech Center
- Overlay

Material Related Cracks

- Durability cracking, commonly referred to as D-cracking, is a distress associated with the freezing and thawing of critically saturated, susceptible coarse aggregate particles in the concrete
- Alkali-aggregate reaction (AAR)
 describes a family of chemical
 reactions between certain
 susceptible aggregates and the
 alkali hydroxides in the concrete,
 which can lead to cracking of the
 concrete matrix and cracking.







D-Cracking

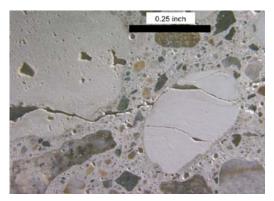
Causes:

Three factors are needed for D-cracking to develop:

- 1) Concrete contains aggregates susceptible to D-cracking in sufficient quantity and size
- 2) Concrete is exposed to sufficient moisture, and
- 3) Concrete is exposed to repeated cycles of freezing and thawing

Prevention

 Effective way of preventing D-cracking is to avoid the use of susceptible aggregates



Fractured Aggregate In D-Cracked
Pavement

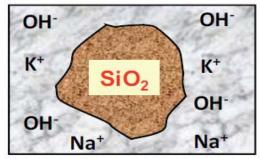


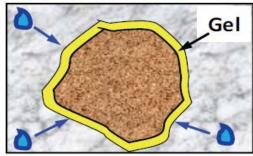
Staining Accompanying D-Cracking at Joints

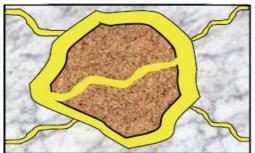


Alkali-Silica Reactivity (ASR)

- ASR is a deleterious reaction between alkalis in the pore solution and reactive silica in aggregate
- Common susceptible aggregates include chert, quartzite, gneiss, and shale, among others







Sequence of ASR Development

Reaction between the alkali hydroxides (Na, K & OH) from the cement and unstable silica, SiO₂, in some types of aggregate.

The reaction produces an alkali-silica gel.

The gel absorbs water from the surrounding paste ...

... and expands.

The internal expansion eventually leads to cracking of the surrounding concrete.



Material Related Cracks - Prevention

ASR Prevention

- Use SCMs
- Avoid susceptible aggregates

D-Cracking Prevention

- Use durable aggregates
- Provide adequate drainage



Required Levels of SCMs to Control ASR (Thomas, Fournier, and Folliard 2013) – Distress Guide

Type of SCM	Total Cementitious Materials
Low-calcium fly ash (<8% CaO; typically Class F fly ash)	20 to 30
Moderate-calcium fly ash (8 - 20% CaO; can be Class F or Class C fly ash)	25 to 35
High-calcium fly ash (>20% CaO; typically Class C fly ash)	40 to 60
Silica Fume	8 to 15
Slag Cement	35 to 65

Material Related Cracks - Treatment and Repairs

Repairs

- Partial-Depth Repair
- Full-Depth Repair/Slab Replacement
- Retrofitted Edge Drains
- Unbonded Concrete Overlay



Maintenance

Joint Filling/Sealing



- Edge Drain Maintenance
- Topical Treatments



Longitudinal and Transverse Cracking









Types

- Volumetric changes (concrete) dry shrinkage/thermal contraction
- Volumetric changes (subgrade) poor soils/drainage
- Settlement and poor support
- Slab length
- Sawing practices
- Traffic loading (design)
- Sympathy cracks



Volumetric Changes in Concrete

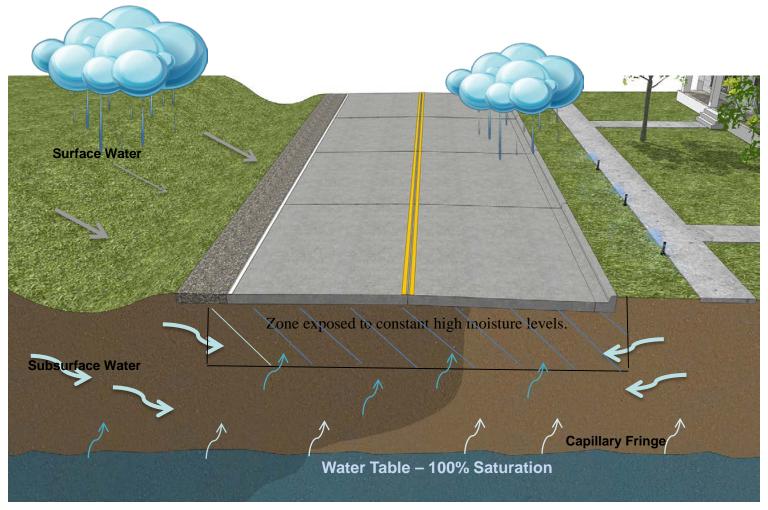
Dry Shrinkage - Increased risk for drying shrinkage cracking are:

- High w/cm ratio.
- High cement contents.
- High-early strength cements.
- High CTE values of the coarse aggregate.

2. Thermal Contraction Cracking

- Temperatures can play a critical role, such that elevated ambient temperatures can induce an early set to the concrete that leads to significant contraction (and potentially cracking) later when it cools.
- sudden change in ambient temperatures.
- paving reaches a higher peak temperature than concrete that is placed later in the day; this is because the peak heat of hydration of the concrete (which typically occurs about 4 to 8 hours after paving) coincides with the hottest part of the summer day

Volumetric Changes in Subgrade



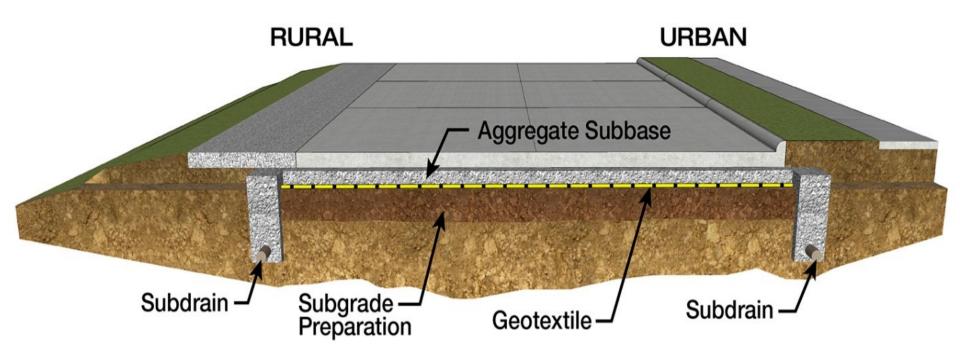


Different Soils; Uniform Support; Drainage Characteristics

Subgrade Treatment Based on Subgrade Conditions

No	Subgrade Conditions	Treatment
1	Varying types of soilMeets M & D testsPasses proof roll test	 Disc and mechanically blend soils (8 in. lifts to 2 ft. depth) for subgrade Compact to 95% standard proctor
2	 Uniformly wet soils Does not pass proof rolling or density test 	 Dry subgrade by disking If drying weather is not available or soils are too wet, utilize quick lime, cement or fly ash
3	Expansive or unsuitable soils	 Chemically stabilize soil with cement (changes the PL and LL to acceptable levels. Remove unsuitable soils and replace with select material

Controlled Drainage System





Longitudinal and Transverse Cracking - Causes and Prevention

Causes	<u>Prevention</u>	
 Excessive slab length 	Follow guidelines, saw to adequate depth	
 Late sawing 	Maximize sawing window, increase labor/equip forces	
 Inadequate saw depth 	Check blades, saw to T/3 on transverse joints	
Traffic loading	Use proper thickness, keep construction traffic away from edges	



Cracking Treatment and Repairs

Repairs

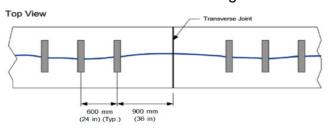
- Full-depth repair
- Cross-stitching/Slot stitching (longitudinal cracking)
- Crack sealing/Filling (if not working crack)
- Diamond grinding

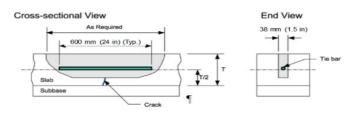
Maintenance

Crack sealing



Slot Stitching





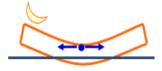


- Concrete slabs undergo nonuniform volumetric changes due to temperature and moisture gradients.
- A gradient is the variation that occurs in temperature and/or moisture from the bottom of the concrete slab to the top.
- The slab is normally colder on the top than the bottom from late at night through mid-morning, resulting in a negative temperature gradient.

Curling and Warping



Positive gradient



Slab less wet on top

Negative gradient

Illustration of Curvature due to Temperature Curling and Moisture Warinig

Curling and Warping

- Jointed concrete pavements can experience upward and downward curvature on a daily basis due to changing temperature gradients (curling) and over time develop upward curvature due to moisture gradients (warping).
- Long-term moisture gradients are almost always negative, with the top of the slab being drier than the bottom; this results in upward warping from the shrinkage that develops in the slab surface as it dries.

Causes - Curling

- Greatest impact on the development of temperature curling is the concrete's coefficient of thermal expansion (CTE).
- As the volume of concrete is predominately aggregate, the aggregate (particularly the coarse aggregate) has a large influence on CTE.
- Limestone aggregates produce concrete with the lowest CTE values.
- Rapid Temperature Changes



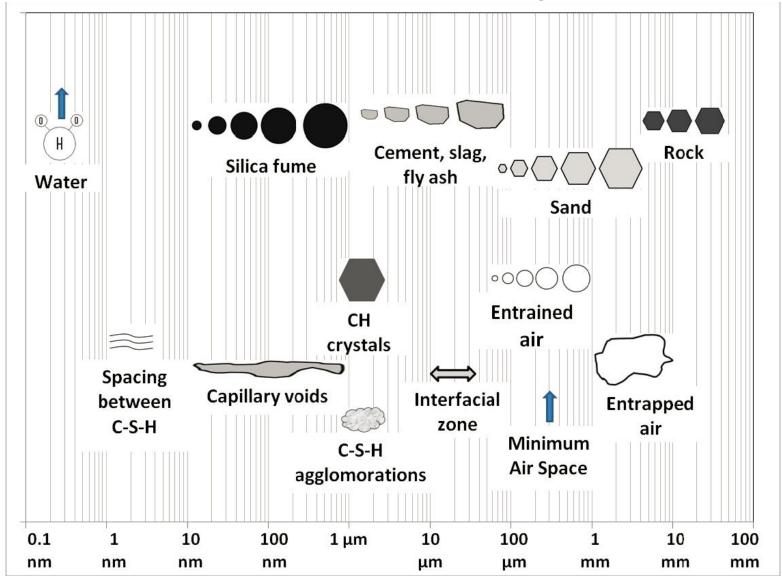
Causes - Warping

- Capillary pores have a large influence on concrete strength and permeability. They have a large influence on the volumetric change that concrete undergoes with changes in moisture. As concrete dries, the pores begin to empty, starting with the largest pores first. In such small pores, as they transition from being full to partially-filled with water.
- In smaller gel pores, the surface tension of the water increases as does the stress pulling the pore walls inward. The result is concrete shrinks as it dries, thus the need for joints to accommodate the shrinkage.
- As dry concrete is re-wetted, the opposite occurs. Empty or partially empty capillary pores take up water, the smallest pores first, until full.



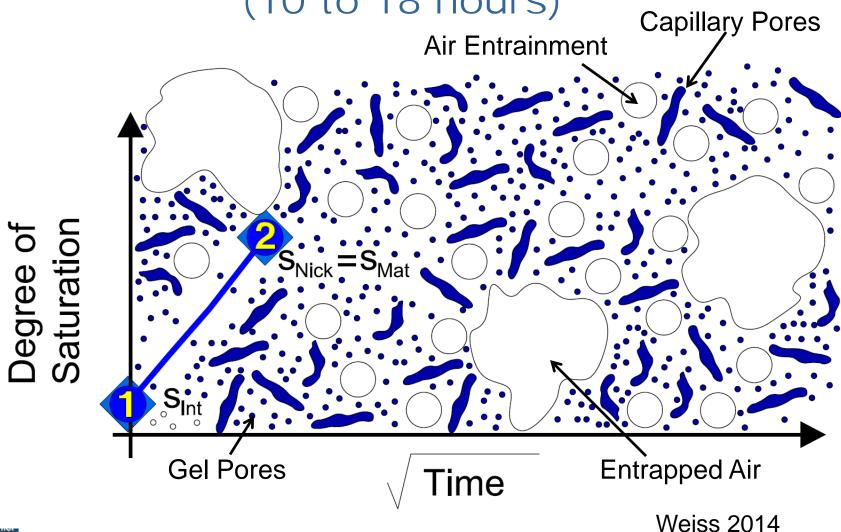
Concrete slabs that undergo cycles of wetting and drying at the surface but remain largely saturated on the bottom will develop upward curvature that will continue to increase for years to come.

Sizes of Concrete Components





Filling of Smallest Pores
-Gel and Capillary Pores(10 to 18 hours)



Curling & Warping Prevention or Mitigation

Distress	Causes	Design	Material Selection	Construction
Slab Curling	 Temperature gradient in slab induces slab curvature and stress High CTE aggreg. contribute to higher levels of curvature and stress 	 Accommodate curling in design through proper joint spacing, load transfer, and base restraint 	When possible, use coarse aggregate with a low CTE	 White pigmented curing compound can help surface cool. Risk of early-age cracking from curling can be reduced by not paving prior to a major temperature fluctuation (i.e. cold front)
Slab Warping	 Moisture gradient in slab induces slab curvature and stress Arid climates, poor drainage, and concrete mixtures susceptible to shrinkage will contribute to higher levels of curvature and stress 	 Accommodate warping in design through proper joints spacing, load transfer, and base restraint Ensure that the pavement is free draining and does not trap water beneath 	Use concrete mixtures with lower potential for drying shrinkage (e.g. lower paste content)	 Minimize evaporation from the concrete surface at an early age; white pigmented curing compounds can help Paving during high temperatures can be a contributing factor; employ hot-weather concreting techniques

Blowups

 Blowups often occur in the heat of the day as expansion results in a buildup of pressure that can be dramatically released as the pavement thrusts upwards and/or shatters.



Blowup in Jointed Concrete
Pavement



Blowup in Jointed Concrete Pavement

Blowups- Concrete Material Factors

- Under most circumstance, the volume of the concrete is never greater than the day it is placed. There are exceptions, however, that sometimes lead to blowups.
- Material factors that can lead to expansion beyond the original concrete volume include moisture and/or temperature effects and expansive reactions.
- As dry concrete is re-wetted empty or partially empty capillary pores take up water, the smallest pores first, until full and the menisci disappear. The pore walls rebound, but not to the same degree as where they started.

Volumetric Expansion Due to Moisture and/or Temperature Effects

- The coefficient of thermal expansion (CTE) defines how a material changes in length with a change in temperature.
- It has been observed that thermal length change is often complicit in the occurrence of blowups, with increasing frequency as temperatures increases.
- Blowups is further increased if the heatwave is accompanied by a significant precipitation which results in saturation and further expansion of the concrete.



Blowups Treatment & Repairs

Blowups are caused by multiple factors that often combine to create excessive compressive stress in the slab

Treatment of Incompressible Materials

 Incompressible material in joints can only be treated by cleaning the joints and resealing them. Procedures for joint resealing can be found in Smith and Harrington (2014).

Repair

 The most common repair strategy is to construct a fulldepth repair.

Spalling



Freeze thaw damage



Saturated joint backer rod damage



Saturated joint with unsound aggregate



Incompressible joint damage



Deflection spalling from heavy vertical loads



Early saw joint raveling



Chloride penetration



Dowel bar misalignment

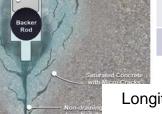


Spalling - Causes

	Distress	Category	Description
		Magnesium & Calcium Chlorides	Deicing chemicals react with Calcium Hydroxide (CH) causing flaking of hardened paste
Spalling (Material or Chemical)	(Material	Freeze Thaw Damage	 Damage to the paste of the concrete from: Poor air entrainment system Saturated concrete joints/cracks Chemical breakdown of the concrete from deicing salts such as calcium and magnesium chloride (Calcium Oxychlorides)
		Thermal Expansion	High coefficient of thermal expansion (CTE) of the aggregate results in higher compressive stresses at the joint or crack.
	Spalling (Physical)	Infiltration Failed Scalant Trapped Water	Infiltration of incompressibles into poorly sealed or unsealed joints.



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Longitudinal Freeze Damage From Backer Rod

Spalling - Deicers

- The formation of Calcium Silicate Hydrate (C-S-H) and Calcium Hydroxide (CH) are the two principal ingredients that mesh into a solid mass forming concrete pavement.
- Magnesium and calcium chloride will react with CH with water at between 32°F and 122°F, depending on the salt concentration.



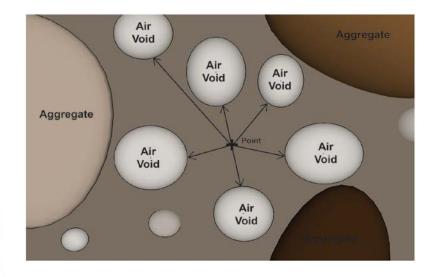
Spalling - Deicers (Calcium Oxychloride)

- This reaction results in the formation of calcium oxychloride which results in flaking (expansion) of the hardened paste causing significant damage particularly in joints.
- Oxychloride expansion can be 3 times greater than freeze-thaw expansion.
- The use of SCM's (fly ash, slag, and silica fume) will reduce formation of calcium oxychlorides by tying up CH
- Use of sealers has also shown the potential to limit interaction between salts and CH



Spalling - Saturation & Poor Air

- Saturation
- Marginal aggregate soundness
- Poor air void system
 - Spacing \leq 0.008 in.

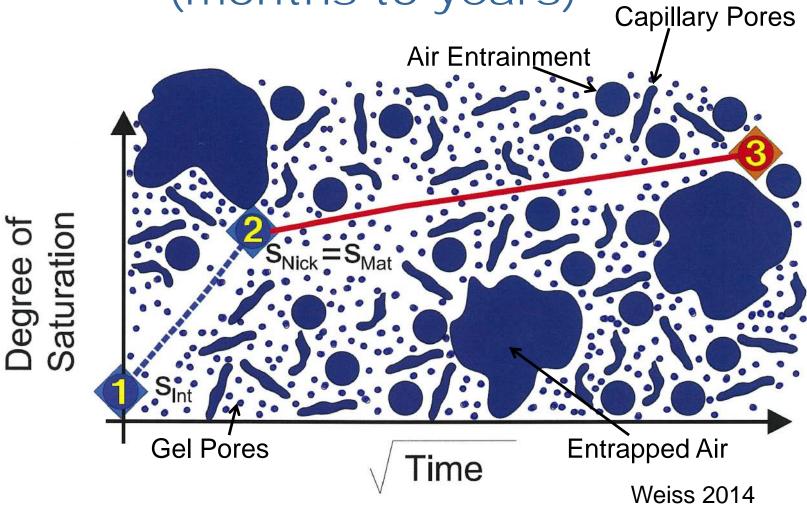




Shadowing

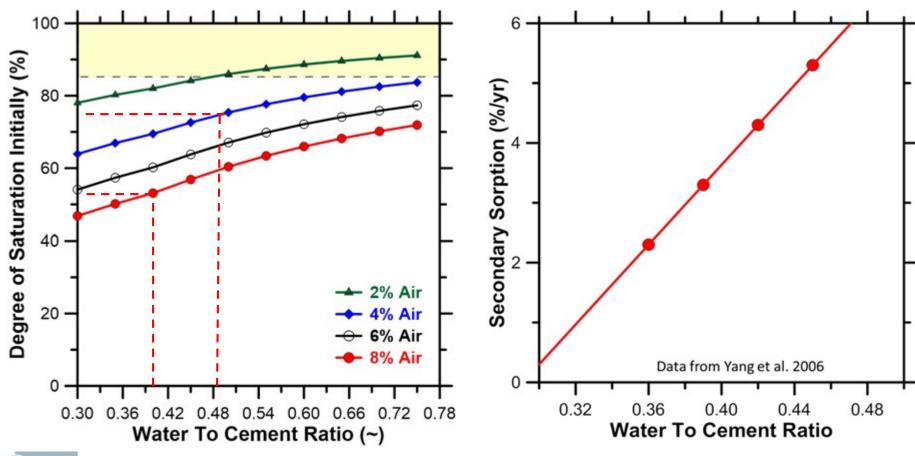


Second- Filling of Larger Pores
-Entrained and Entrapped Air(months to years)





Critical Saturation Rates





Spalling - Summary of Prevention

- Prevent saturation
- Reduce concrete permeability
 (Use fly ash to tie up CH)
- Ensure adequate air entrainment



Image of a Super Air Meter (SAM) (Photo credit Tyler Ley)



Spalling - Treatment and Repairs

Repairs

- Partial-Depth Repair
- Full-Depth Repair/Slab Replacement
- Retrofitted Edge Drains
- Unbonded Concrete Overlay

Maintenance

- Winter maintenance
- Maintaining sealed joints
 - Maintaining sub-drain systems

Settlement and Heaves









Causes

- Inadequate base compaction
- Consolidation of support layers under traffic
- Subgrade soil movement
- Loss of support (contamination of base layers)
- Frost heave
- Expansive soils



Settlement and Heaves Prevention

Material Selection

Causes

Design

Construction

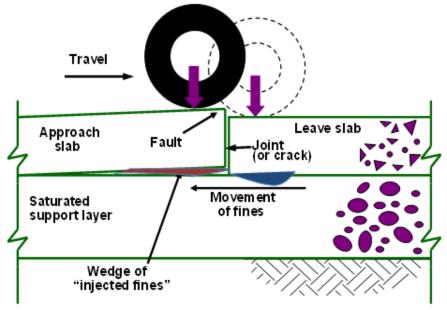
Preventive

				Maintenance	
Poor consolidation	Characterize soil based on engineering properties	Limit % fines to 10% (passing #200 sieve)	Proper compaction	Routine joint sealing Periodic maintenance of drainage system including cleanout of outlets	
Volume changes in soil	Compaction specification based on optimum M & D	Uniform soil, blend, granular subbase or chemical treatment	Uniform soil, blend, granular subbase or chemical treatment		
Excessive moisture	Provide drainage Consider chemical stabilization	Use drainable subbase	Dry soils Compact at optimum M & D		

Faulting









Faulting - Causes

Distress	Item	Description	
Faulting (Physical)	Load Transfer Loss	Failure of aggregate interlock or mechanical devices that transfer load across pavement joints and cracks	
	Pumping	Longitudinal and transverse cracks and unsealed joints which allow water intrusion and lead to future faulting including loss of load transfer	
	Loss of Seal Integrity	Portal for intrusion of water into the grade	
Faulting (Material or	Poor Aggregate	Poor quality coarse aggregate leads to early loss of load transfer due to low shear capacity Aggregate particles deteriorate resulting in loss of support	

support

or

Chemical)

Soundness

Faulting - Treatment and Repairs

Repairs

- Dowel Bar Retrofit
- Bonded Concrete Overlay (if faulting is less than less than 3/8" faulting or Unbonded Concrete Overlay for 3/8" to 5/8")

Maintenance

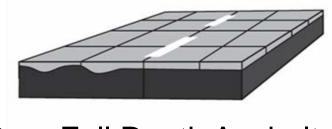
Diamond Grinding



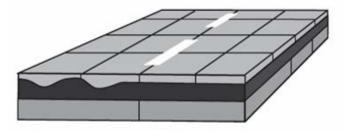




Bonded Concrete Over Asphalt BCOA



Over Full Depth Asphalt



Over Composites



Corner Cracks

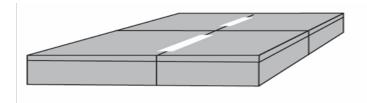


Longitudinal Cracks



Blowup Crack

Bonded Concrete Over Concrete BCOC





Multiple Cracks

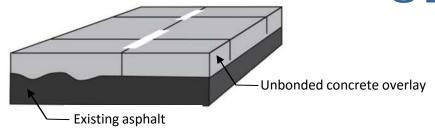


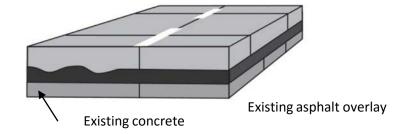
Longitudinal Cracks



Reflective Cracks

Unbonded Concrete Over Asphalt **UBCOA**





Over Full Depth Asphalt

Over Composites





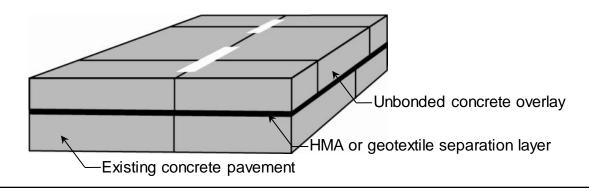


Longitudinal Cracks Transverse Crack

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Faulting

Unbonded Concrete Over Concrete UBCOC





Longitudinal Cracks

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Reflective Crack



Faulting

Summary of Overlay Distresses

Prevention or Mitigation

Distress	Causes	Design	Material Selection	Construction
1. Longitudinal cracking in tied shoulders and widened sections	 Differential movement (heaving) of materials underlying shoulders and/or widened sections UBCOA Tie-bar placed at the bottom of the concrete slab Poor subbase drainage of widening or shoulder 	 Tie-bar placed at neutral axis if possible Utilize structural fibers in lieu of tiebars Provide for drainable subbase under shoulder 	• Maximum tie-bar size of #4	 Chair or insert bars to specified tolerances QC checks to confirm tiebar location behind the paver Saw all contraction joints to a depth of T/3 except for BCOC which is full depth
2. Longitudinal cracking in wheelpath	 Inadequate slab thickness Debonding for BCOA & BCOC due to curl/warp or deteriorated existing pavement Improper slab dimensions Inadequate load transfer 	 Adequate overlay thickness Appropriate slab dimensions Load transfer devices when thickness is greater than 7" Avoid placing longitudinal joints in the wheelpath 	 Structural fibers may increase load transfer in thin overlays Existing HMA Asphalt should be 	 Control the temperature and moisture of the HMA at time of paving Adequate curing Sawcut longitudinal joints to T/3 Limit construction traffic on remaining HMA to prevent damage Provide clean existing pavement

Summary of Overlay Distress

		Prevention or Mitigation		
Distress	Causes	Design	Construction	
3. Transverse cracking	Reflective cracking of underlying thermal cracks	Set profile grade to assure adequate thickness of existing HMA asphalt remains after milling	 Securely anchor baskets Adjust anchoring for non-uniform conditions Do not cut shipping wires on the baskets Sawcut transverse joints to T/3 except for BCOC which are full depth 	
4. Blowups	 Undeployed joints non-uniform subbase friction Incompressibles in the joints Expansion due to heavy rain, excessive heat or both 	 Appropriate slab dimensions for the design thickness Specify sealing joints to prevent incompressibles from filing the joints A full-depth transvers sawcut for BCOC Ensure that the underlying pavement is ell drained 	 Saw cut joints to T/3 except for BCOC which are full depth Seal all joints 	

THANK YOU!

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