

# Phase I (Laboratory): Investigation of Soil Stabilization Alternatives – Texas SH 130

December 2008

## Final Report



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<b>16. Abstract</b>  Portland cement, hydrated lime, slag, class C and F fly ash, and a few nontraditional chemical stabilizers were evaluated in the laboratory for their effectiveness in improving strength and reducing swell potential of high-plasticity clay materials sampled at the proposed SH 130 project site in Texas. In addition to high plasticity index values, soils in the proposed construction area reportedly contain variable amounts of sulfate content that may lead to deleterious expansion due to secondary mineral growth. Over 900 test specimens from the project site were prepared and tested.  The goal of the laboratory phase of this project was to provide recommendations for conducting field test strip evaluations of the select portland cement stabilizers or combinations of Portland cement and other stabilizers. Unconfined compressive strength, volume change, pH, soil classification, X-ray analysis, and scanning electron microscopy methods were used in this analysis. An unconfined compressive strength of 100 psi, a maximum volume change of 2%, and a decrease in the plasticity index were used as evaluation criteria for determining suitable stabilizers for field trials.  The following cement-based stabilization mixtures would meet the laboratory-established criteria: 6% Cement Type I/II or Type V, 4% Cement Type I/II + 4% Fly ash Class C, 4% Cement Type I/II + 4% Fly ash Class F, and 3% Cement Type I/II + 3% Fly ash Class C or F. Further evaluation by field testing, with several months of evaluation including in-situ testing, sampling, and in-ground instrumentation monitoring, is recommended for Phase II. A testing plan for evaluation is presented in this report. The outcome of the field investigation would be conclusive performance results for the recommended stabilizers in terms of strength, stiffness, and volume change. It is further recommended that the stabilization construction methods be evaluated as part of the field evaluation. This would specifically include variable mixing rates and use of intelligent compaction technology with GPS mapping capabilities.			
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## INTRODUCTION

This report summarizes the results of a laboratory study to evaluate the effectiveness of portland cement, hydrated lime, slag, class C and F fly ash, and other nontraditional chemical stabilizers for improving strength and reducing swell potential in high-plasticity clay materials sampled from the proposed SH 130 project site between Austin and San Antonio, Texas. Soils in this project area are generally characterized by high plasticity index values and high clay contents, as well as variable and isolated high concentrations of soluble sulfate.

The laboratory study was performed on materials received in September 2007. Unconfined compressive strength, volume change, pH, soil classification, X-ray analysis, and scanning electron microscopy (SEM) methods were used in this analysis. A target unconfined compressive strength of 100 psi and a maximum volume change of about 2% were selected as evaluation criteria to determine suitable stabilizers.

The goal of the laboratory phase of this project was to provide recommendations for conducting field test strip evaluations of the portland cement stabilizers or combinations of the portland cement stabilizers. Most of the stabilizer combination analysis focused on the performance of cement types I/II and V by themselves and in combination with other stabilizers. Findings suggest that the following cement-based stabilization mixtures would meet the laboratory-established criteria are recommended for field trials:

- 6% Cement Type I/II or Type V
- 4% Cement Type I/II + 4% Fly ash Class C
- 4% Cement Type I/II + 4% Fly ash Class F
- 3% Cement Type I/II + 3% Fly ash Class C or F

For these mixtures, the plasticity index (PI) values are substantially reduced, ranging initially from 31 to 49 for the untreated soil to 8 to 23 for the treated soils.

To move forward with evaluating these stabilization alternatives, it is recommended that field test sections be constructed, tested, and evaluated for a period of several months. For soils that contain high soluble sulfate content, these treatments may not be adequate and may require progressive retreatment or simple removal and replacement with other materials. Other stabilization mixtures, not listed above, also meet the laboratory criteria, but are not suggested for field trial unless needed as a benchmark. For the four mixture types listed above, the field test sections would provide conclusive information to verify the laboratory results. If the field trials confirm the laboratory results, a cement-fly ash based stabilization mixture would provide an alternative to traditional methods and has the advantage of potentially reducing costs for some applications. Recommendations for stabilizers and the field test strips are provided in this report.

## **TEST METHODS**

This research used several methods to evaluate the soil stabilizers' effect on the mechanical and chemical properties of the soil. Based on the nearest city from the soil sample location, as well as measured engineering properties, materials are designated herein as soil groups "S", (Sequin) and "L" (Lockhart). It should be noted, the "S" group was determined to be from the Wilcox formation, while the "L" was determined to be from the Navarro formation.

### **Atterberg Limits**

The untreated soils and combinations of soil and stabilizer were tested for liquid limit (LL), plastic limit (PL), and plasticity index (PI) according to ASTM D4318. Untreated and treated soils were mellowed for 24 hours prior to testing. Treated soils were tested following a 28 day cure period at ambient conditions. Some tests were performed at cure periods up to a few days longer than 28 days due to schedule challenges. It is not expected that the additional cure periods had a significant impact on results.

### **Particle Size Analysis**

Soils particle size analysis was conducted in accordance with ASTM D422. Clay size was set at 0.002 mm. Appendix A provides the particle size analysis results.

### **Moisture Density Relationship**

Optimum moisture contents were used in the mix designs as a reference for the moisture conditions of the materials. Generally, the materials were batched at moisture contents above optimum to ensure stabilizer hydration and reduce swell potential. All as-received soils were processed through a No. 4 sieve, moisture-conditioned and mellowed for 24 hours, and compacted into specimens using the Iowa State University 2x2 apparatus (O'Flaherty 1968), which simulates standard Proctor compaction energy as defined in ASTM D698. The untreated samples used for defining the moisture-density relationship were also tested for swell and unconfined compressive strength. The advantage of the 2x2 apparatus is that it facilitates efficient sample production, which can be advantageous for testing many combinations of stabilizers.

### **Soluble Sulfate Determination**

Soils were tested for soluble sulfate content according to Tex-145-E, Determining Sulfate Content in Soils Using Colorimetric Method, and Tex-146-E, Conductivity Test for Field Detection of Sulfates in Soil. Tex-145-E testing was performed using an Orion AQUAfast II Turbidity Colorimeter. Samples containing high concentrations of sulfate were diluted to lower concentrations than those specified to provide measurements within the readability concentration of the measuring equipment. Tex- 146-E testing was conducted using an Accumet XL20 meter with a glass body, four-cell/ATC conductivity probe with a nominal cell constant of 1.0 cm<sup>-1</sup>.

While this method can be considered a field test for determining sulfate, previous lab testing has resulted in quantitative correlations between conductivity and sulfate concentration (Harris et al. 2002). These correlations were used in this analysis.

### **pH Determinations**

The pH values for the soil materials were analyzed using a LaMotte pHPlus meter calibrated with standard solutions of pH 7, 10, and 12. A 1:1 ratio technique described in “Recommended Chemical Soil Test Procedures for the North Central Region” was performed on natural, as-received materials (Watson and Brown 1998). Additional pH tests were performed during the planning of batch proportions and on 90 day ambient cured samples using the 1:1 ratio technique. The method of pH testing involved adding 25 g of distilled water to 25 g of testing material, hand mixing for 1 minute, allowing the material to stand undisturbed for 15 minutes, and measuring the pH. For the batch design, additional stabilizing agents were added in established proportions to see variations in pH values based upon changes in constituent concentration.

### **2x2 Sample Preparation**

Material proportions for producing 2 inch x 2 inch specimens were measured gravimetrically and mixed thoroughly, mellowed for 15 minutes (for compaction delay), and then compacted using the Iowa State University 2x2 apparatus. Over 900 specimens were produced. A total of 24 specimens were prepared for each batch. Six specimens each were tested at 7 days (ambient temperature), 7 days (38°C), 28 days (ambient temperature), and 90 days (ambient temperature). Ambient temperature was approximately 21°C. Three of these samples were tested for strength, and three specimens were subjected to volumetric swell and subsequent strength tests. Due to the low amount of material containing a high sulfate content, four samples were prepared for select stabilizers, and one each was tested at 7 day ambient temperature, 7 plus 3 day ambient temperature swell, 28 plus 3 day ambient temperature swell, and 90 plus 3 day ambient temperature swell. All batch specimen measurements are included in Appendix B. The averages of triplicate test samples are summarized later in this report.

### **Unconfined Compressive Strength**

Strength tests were performed in general accordance with ASTM D2166-00 using an ELE Versa-Loader load frame with digital displacement and load display. The loading rate was 0.05 in. per minute.

### **Volumetric Swell Determination**

Volumetric swell measurements were recorded for each group of three 2x2 samples using methods based on ASTM D559. Samples were placed on porous stones and protected from direct water contact in a 100% relative humidity room for three days to allow expansion/swelling. Height and diameter measurements were taken with digital calipers, and average values were recorded. The final values for all three samples were combined for the final reported results.

## **X-Ray Analysis**

X-ray analysis of the soil and stabilizing materials was conducted at the Material Analysis and Research Laboratory (MARL) at Iowa State University (ISU). X-ray fluorescence (XRF) measurements were taken using a Philips PW 2404 X-ray spectrometer using Rh X-ray tubes operated at 3,600 watts. Fused disk standards from the National Institute of Standards and technology (NIST) were used. Materials were analyzed in vacuum mode. The sulfur content of the soil was analyzed on loose powders in helium mode using calcium sulfate-based standards from the American Chemical Society (ACS). X-ray diffraction (XRD) analysis was performed using a Siemens D 500 X-ray diffractometer with Cu X-ray tube source operated at 50 kV and 27 mA using medium-resolution slits.

The cement and fly ash samples were ignited to a constant mass and then fused into glass disks for XRF analysis. The lime and slag samples were pressed into pellets for XRF analysis. All stabilizing samples were ground in an agate mortar and pestle prior to XRD analysis. Soil materials were oven-dried and ground for two minutes in a SPEX shatter box to produce a fine powder for XRD analysis. XRD specimens were back-packed to avoid preferred orientation. Selected samples were equilibrated with water vapor (100% relative humidity, 24 hour treatment) at room temperature ( $23\pm 2^\circ\text{C}$ ) in an attempt to rehydrate the oven-dry soils. Then the samples were allowed to sit at ambient temperature and humidity ( $23\pm 2^\circ\text{C}$ , 25% to 35% relative humidity) for about two hours prior to XRD analysis. A companion sample was treated with ethylene glycol to check for expansive clay minerals. The glycol-treated specimens (and a single water-treated specimen) were smeared into the XRD holder, which tended to cause preferred orientation of the basal planes of the clay minerals.

## **Thermal Analysis**

The lime sample was subjected to thermogravimetric analysis (TGA) to determine loss-on-ignition (LOI) and to estimate the calcium hydroxide and calcium carbonate contents. The material was ground in an agate mortar prior to testing. A TA Instruments TGA, model number 2950, was used at a temperature range of 25 to 1,000°C, with a 40°C increase per minute. The resolution was 5°C, and the sample size was 25 mg.

## **Scanning Electron Microscopy**

Additional testing was conducted in the MARL facility to obtain SEM images of soils and soil/cement mixtures to look for sulfate-rich material morphology as well as to perform spatial elemental analysis. A Hitachi S-2460N variable pressure SEM equipped with secondary electron and backscattered electron detectors, a motorized stage, and an Oxford Instruments ISIS system with energy-dispersive spectrometers (EDS) for quantitative elemental analysis was used for SEM analysis. Backscatter and secondary electron images were acquired.



## **Cation Concentration Organic Content and Total Carbon Content**

Cation concentration, organic matter, and total carbon content measurements were conducted at the Iowa State University Soil and Plant Analysis Laboratory using “Recommended Chemical Soil Test Procedures for the North Central Region” (Warncke and Brown 1998).

## MATERIALS

### Soils

Soils from three locations were received for investigation. Soils were grouped based on similar physical and chemical properties. Table 1 provides the material inventory.

**Table 1. Materials received**

Group	Material Description	Sample #
<b>S</b>	Segment 6.2 (Wilcox Formation) Light Brown/Orange Grey Mottled	1
		1
		2
	Segment 5 (Navarro Formation) Depth 1 of 5 feet Black/Trace Brown	3
		4
<b>L</b>		5
	Segment 5 (Navarro Formation) Depth 1 to 7 feet Black/Trace Brown	6
		7
	Segment 5 (Navarro Formation) Depth 1 to 5 feet Black/Trace Brown	8
	Segment 5 (Navarro Formation) Depth 1 to 7 feet Black/Trace Brown	9
<b>L Sulfate</b>	Segment 5 (Navarro Formation) Black/Trace Brown	1

There were two groupings of soils based on the testing performed and only one container of material (35 lbs at in situ moisture content) that contained high sulfate content. Table 2 summarizes the two soil types investigated, as well as the properties of the single sample that had high sulfate content.

**Table 2. Properties of soil materials tested**

Soil Designation	S	L	L (high sulfate)
PI	31	49	45
LL	51	74	73
Dry Unit Weight (pcf)	111.0	92.5	100.0
Optimum Moisture Content (%)	17.5	28.0	25.8
Clay Content (0.002 mm) %	40	58	—
Volumetric Swell (%)	18	12	14
Initial Moisture Content (%)	16.8	28.3	27.0
Final Moisture Content (%)	27.5	39.2	43.8
ppm K	185	434	328
ppm Ca	3010	10088	26285
ppm Mg	670	147	653
ppm Na	396	38	715
pH	5.7	8.7	7.8
% Carbon	0.4	2.5	0.8
%Organic Matter	0.7	4.5	1.4
Sulfate TX 145-E (ppm)	< 100 (low)	< 100 (low)	30320 (high)*
Sulfate TX-146-E (ppm)	< 100 (low)**	700 (low)**	31300 (high)**

\* Sample diluted to 1:200, a dilution outside of standard used to obtain quantifiable amount in equipment measurement range; \*\* determined from correlation Harris (2002)

X-ray analysis was conducted on soil samples for SO<sub>3</sub> content (actually bulk sulfur content, expressed as %SO<sub>3</sub>). The bulk sulfur values, shown in Table 3, were generally low (less than 0.1% for the majority of samples). The samples denoted as MM were analyzed in triplicate to estimate the precision of the determinations. The bias in the method could not be determined due to a lack of appropriate standard reference materials. Calibration standards were typically only accurate to two decimal places, so the precision of the method greatly exceeded its accuracy. The sample denoted as RR (highest sulfur content) was also analyzed using the fused disk technique to provide a better estimate of its bulk chemical composition (see Table 4).

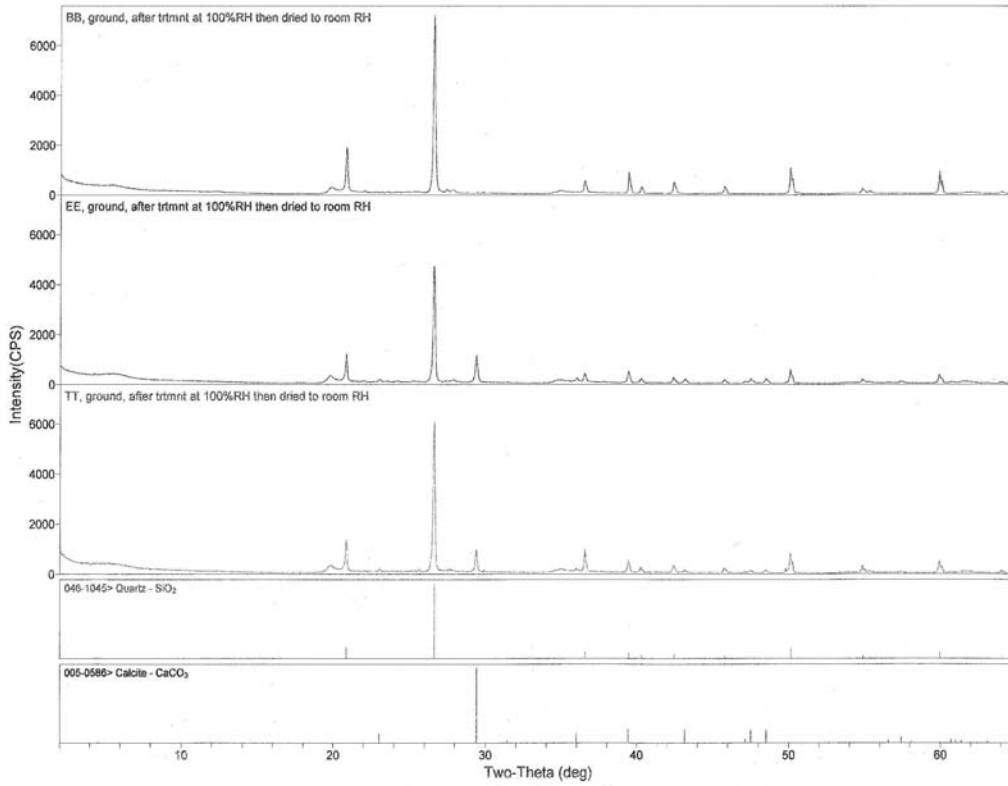
**Table 3. Summary of XRF assays performed**

MARL Designation	Sample Desc.	Data type	S peak	S bg1	S net kcps	C SO3, %	kcps/%	Rough est. SO3 calculated	Regression. SO3 calculated
STD B-670+FILM	—	Raw kcps	162.6269	0.6014	162.53	5.63	28.87	<b>5.63</b>	5.64
STD B-243+FILM		Raw kcps	78.1532	0.3466	78.05	3.43	22.76	<b>3.45</b>	3.45
STD B-15-16+FILM		Raw kcps	77.3196	0.3436	77.22	3.43	22.51	<b>3.42</b>	3.42
AA	S1	Raw kcps	0.7165	0.1002	0.62	—	—	<b>0.027</b>	0.033
BB		Raw kcps	0.601	0.0941	0.50	—	—	<b>0.022</b>	0.026
CC		Raw kcps	0.4863	0.0917	0.39	—	—	<b>0.017</b>	0.020
DD	L1	Raw kcps	1.733	0.0996	1.63	—	—	<b>0.072</b>	0.086
EE		Raw kcps	9.3689	0.1268	9.27	—	—	<b>0.410</b>	0.481
FF	L2	Raw kcps	0.9258	0.0987	0.83	—	—	<b>0.037</b>	0.044
GG		Raw kcps	1.0327	0.0991	0.93	—	—	<b>0.041</b>	0.049
HH	L3	Raw kcps	0.6838	0.0991	0.58	—	—	<b>0.026</b>	0.031
II		Raw kcps	0.9012	0.0976	0.80	—	—	<b>0.035</b>	0.042
JJ	L4	Raw kcps	2.882	0.109	2.78	—	—	<b>0.123</b>	0.146
KK		Raw kcps	0.6401	0.0983	0.54	—	—	<b>0.024</b>	0.029
LL	L5	Raw kcps	2.847	0.0997	2.75	—	—	<b>0.122</b>	0.144
MM		Raw kcps	0.5726	0.0963	0.47	—	—	<b>0.021</b>	0.025
NN	L6	Raw kcps	3.0763	0.1046	2.98	—	—	<b>0.132</b>	0.156
OO		Raw kcps	0.7043	0.098	0.60	—	—	<b>0.027</b>	0.032
PP	L7	Raw kcps	5.1306	0.1137	5.03	—	—	<b>0.223</b>	0.263
QQ	L8	Raw kcps	1.47	0.105	1.37	—	—	<b>0.061</b>	0.072
RR	L Sulfate	Raw kcps	220.4742	0.7025	220.37	—	—	<b>7.63</b>	6.22
SS	L9	Raw kcps	0.6129	0.0996	0.51	—	—	<b>0.023</b>	0.027
TT		Raw kcps	0.7135	0.0955	0.61	—	—	<b>0.027</b>	0.032
UU		Raw kcps	0.576	0.0992	0.48	—	—	<b>0.021</b>	0.025

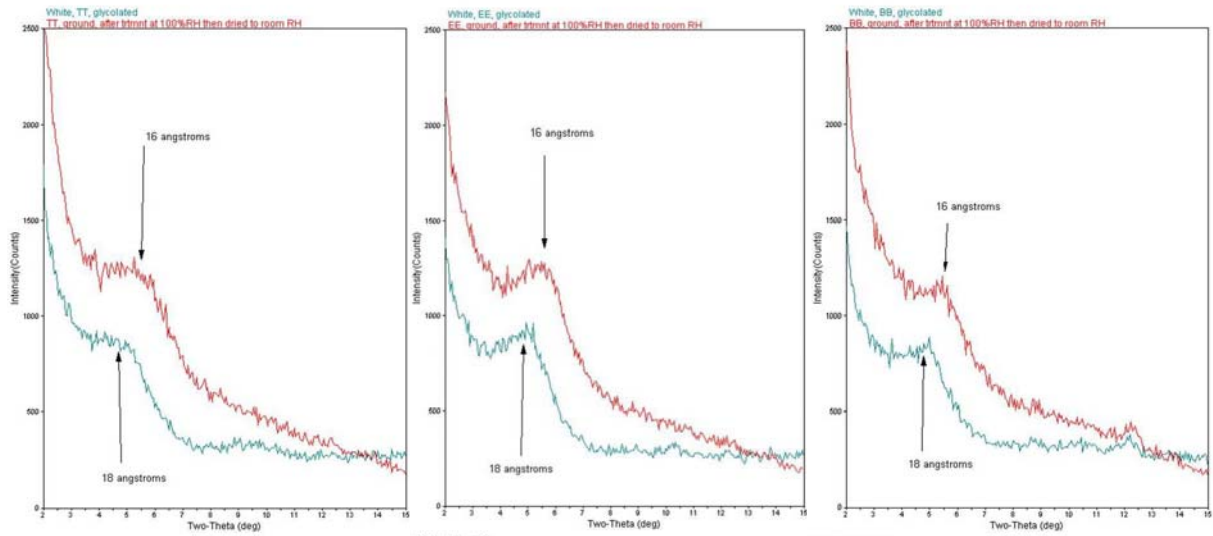
**Table 4. XRF Analysis of RR, high sulfur soil, fused disk preparation**

Oxide or constituent	Mass%
Na <sub>2</sub> O	0.66
MgO	1.48
Al <sub>2</sub> O <sub>3</sub>	11.39
SiO <sub>2</sub>	51.77
P <sub>2</sub> O <sub>5</sub>	0.08
SO <sub>3</sub>	10.94
K <sub>2</sub> O	1.24
CaO	10.64
TiO <sub>2</sub>	0.60
Fe <sub>2</sub> O <sub>3</sub>	3.92
SrO	0.04
Mn <sub>2</sub> O <sub>3</sub>	0.05
BaO	0.01
Moisture content (at 110° C)	0.35
Loss-on-ignition (at 950 ° C)	6.05

Sample RR (L Sulfate 1) and six other samples, were subjected to XRD analysis before and after selected treatments, as shown in Figure 1. The clay mineral, with a d-spacing of about 15 to 16 angstroms from the air-dry soil samples, tended to expand to about 18 angstroms when treated with ethylene glycol, as shown in Figure 2. This expansion indicated the presence of a swelling clay mineral. The overlays in the figure show the glycolated treatment for expansive mineral investigation, which indicates the presence of swelling clays. Sample RR was treated with water. This caused the clay basal plane to expand to 20 angstroms and caused the bassanite to hydrate to gypsum. Complete soil diffractogram results are provided in Appendix C.



**Figure 1. Diffractograms of received soils (“BB” is S soil type, “EE” and “TT” are L soil type)**



**Figure 2. Diffractograms of received soils, glycolated (“BB” is S soil type, “EE” and “TT” are L soil type)**

## SEM Summary of Soils Received

The as-received materials in the identified testing groups were investigated using SEM techniques. The images provide particle shape, texture, and morphology, while the X-ray analysis provided elemental composition. Because many of the minerals are clays, a high concentration of silicon and aluminum was recorded. The presence of gypsum was noted and mapped using EDS in the high sulfate soil (Figure 3). In the figure, sulfur (SKa, 21) and calcium (CaKa, 22) show the presence of gypsum, and light areas indicate spatial regions of high intensity for the noted element. Figure 4 validates the high concentration of calcium, sulfur, and oxygen in the assumed  $\text{CaSO}_4$  particle, showing a high intensity of counts for these elements. The inset image in Figure 4 shows the area of analysis. Appendix D contains the SEM images acquired.

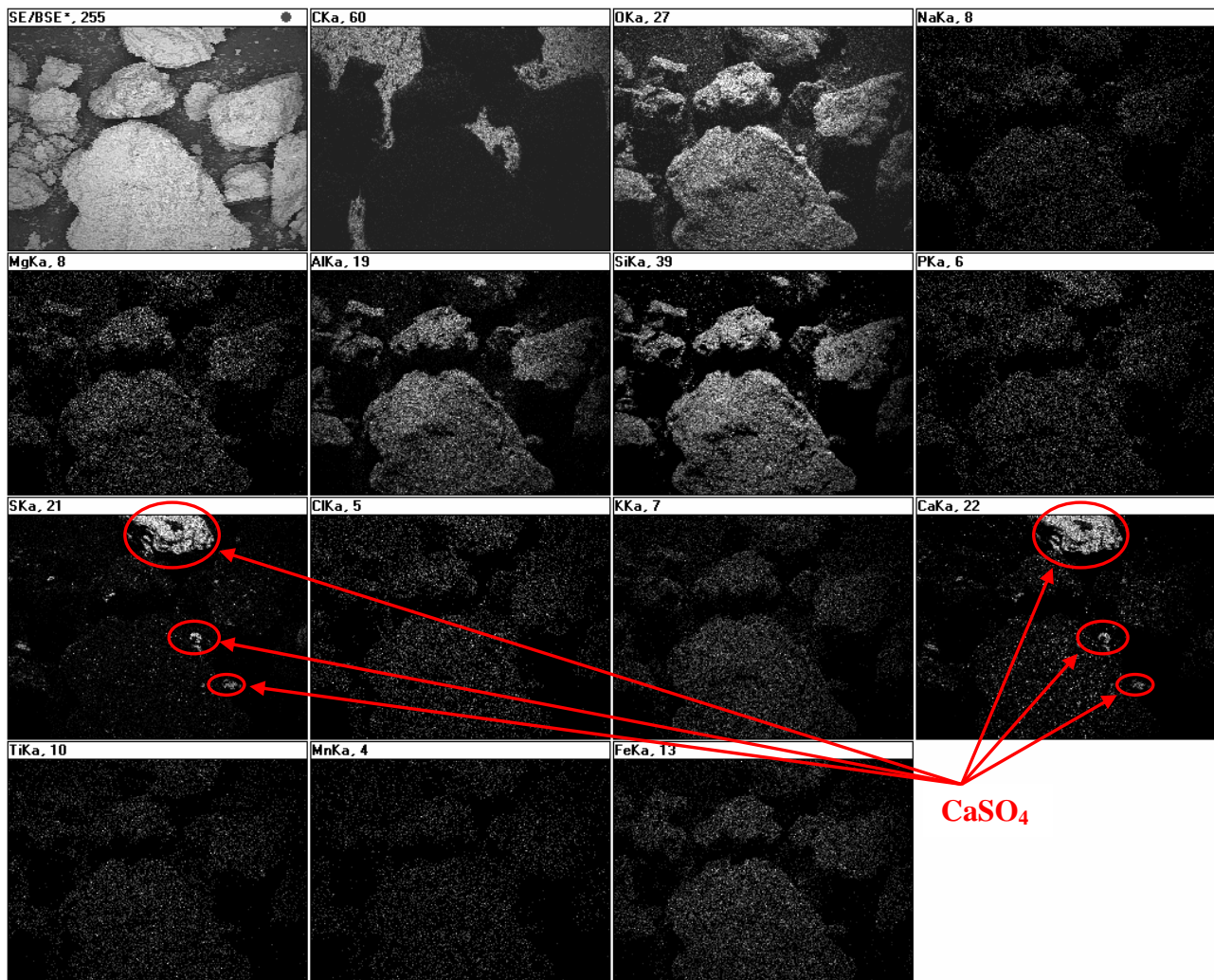
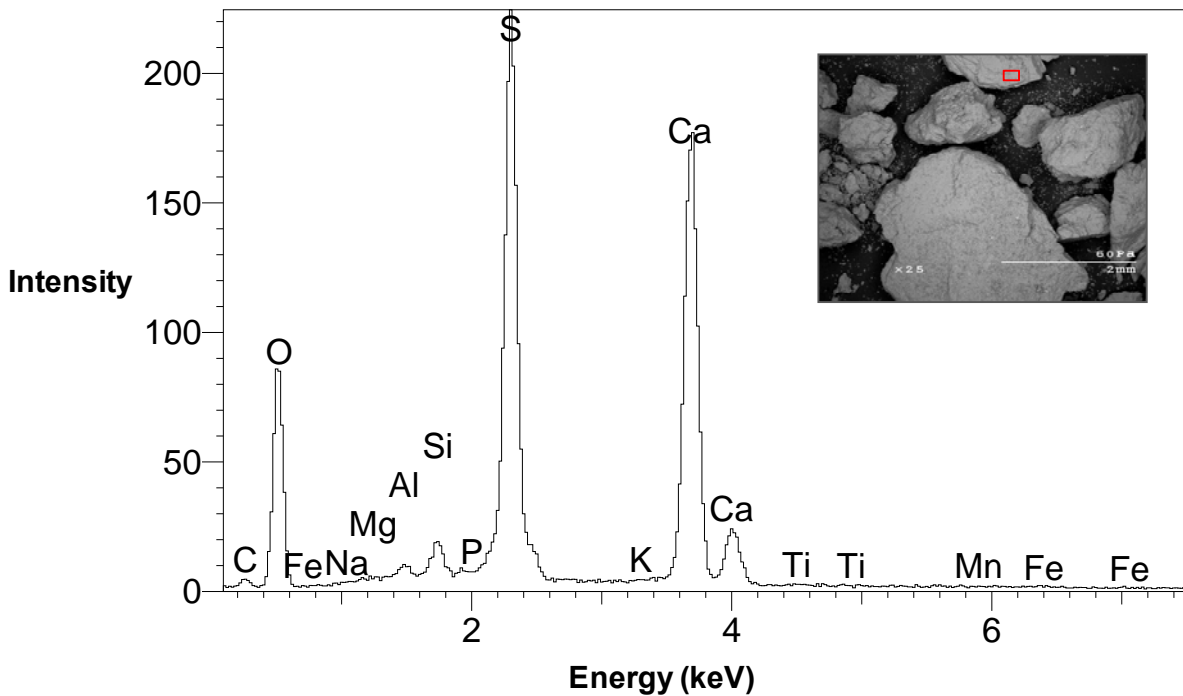


Figure 3. X-ray EDS maps of high sulfate content soil



**Figure 4. EDS intensity counts, showing high concentrations of oxygen, sulfur, and calcium**

### Stabilizers

Six stabilization materials were provided for batching, as summarized and described in Table 5. The results of chemical assays show reasonable values for the corresponding materials, as summarized in Tables 6 and 7. In Table 6, the fly ash assays are expressed on a dry basis; the samples were ignited at 750°C prior to analysis. The cement assays in the table are expressed on an as-received basis; the samples were ignited at 950°C prior to analysis. In Table 7, the slag and lime assays are expressed on an as-received basis, and the CaO content of the lime was calculated by difference from 100%. Additionally, XRF and XRD analyses were performed on the stabilizers. Diffractograms were in general agreement with the chemical assays that were acquired. Appendix E contains the complete set of stabilizer diffractogram data.

**Table 5. Summary of stabilizers received**

Material Type	Description
Cement Type I/II	Cemex, New Braunfels, TX
Cement Type V	Cemex Type V, Odessa, TX
Fly Ash Class C	Mineral Resource Technologies, Inc.
Fly Ash Class F	Mineral Resource Technologies, Inc. Petersburg, IN.
Lime (Hydrated)	Chemical Lime Company Fort Worth, TX
Slag	CEMEX



**Table 6. Summary of fly ash and cement assays**

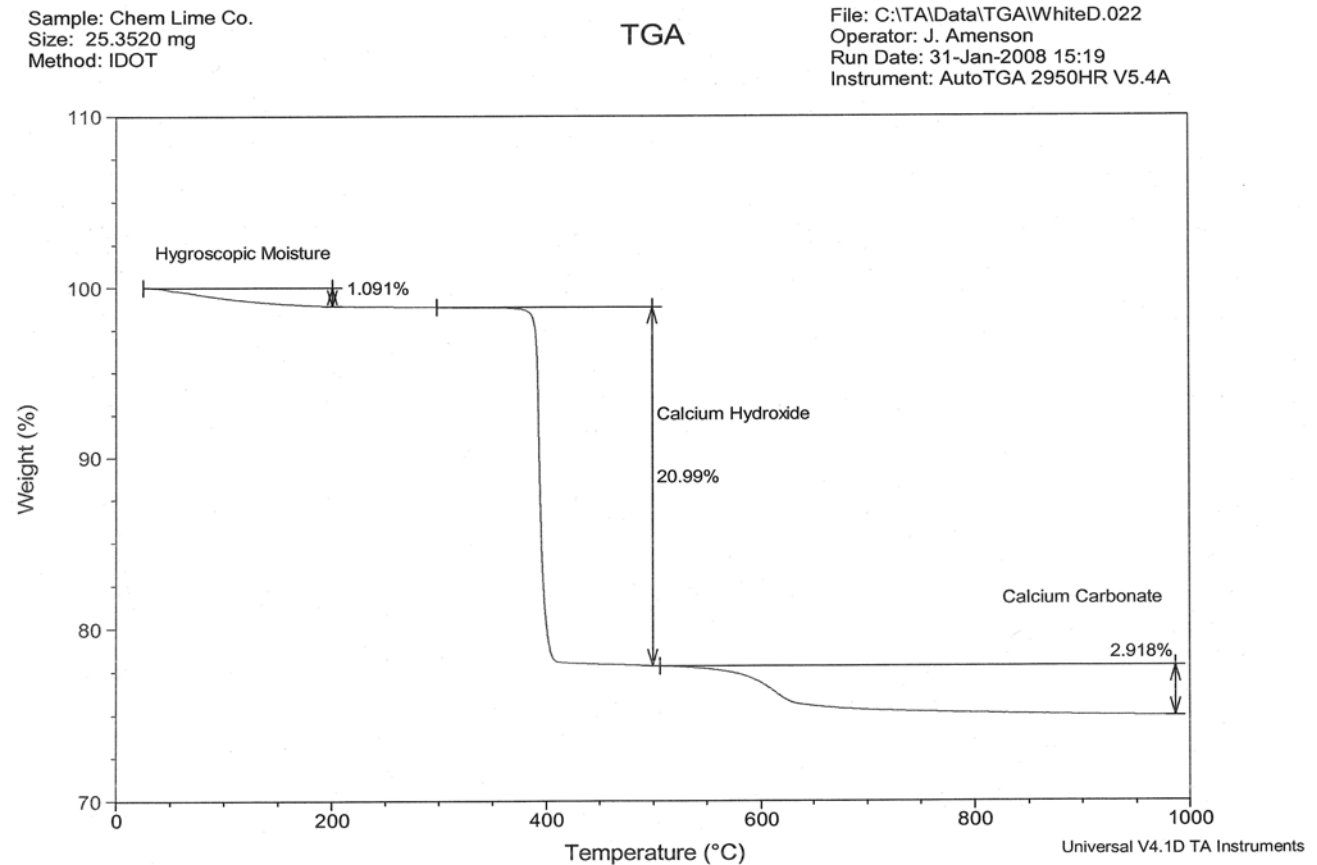
Oxide or constituent, mass%	MRT F ASH	MRT C ASH	Cement New Braunfels-Type I/II	Cement Odessa- Type V
Na <sub>2</sub> O	0.49	1.64	0.19	0.33
MgO	0.85	4.9	1.09	0.72
Al <sub>2</sub> O <sub>3</sub>	22.7	19.5	4.49	4.16
SiO <sub>2</sub>	46.1	36.1	20.1	21.3
P <sub>2</sub> O <sub>5</sub>	0.21	1.10	0.07	0.06
SO <sub>3</sub>	0.32	1.46	2.90	3.91
K <sub>2</sub> O	2.41	0.47	0.59	0.22
CaO	1.84	26.2	64.4	63.3
TiO <sub>2</sub>	1.03	1.65	0.21	0.17
Fe <sub>2</sub> O <sub>3</sub>	20.8	5.74	3.37	4.39
SrO	0.05	0.42	0.05	0.07
Mn <sub>2</sub> O <sub>3</sub>	0.03	0.03	0.04	0.05
BaO	0.06	0.8	not measured	not measured
LOI	1.88	0.19	2.52	1.03
Calculated via ASTM C 150				
C3S	not applicable	not applicable	66	51
C2S	not applicable	not applicable	8	23
C3A	not applicable	not applicable	6	4
C4AF	not applicable	not applicable	10	13

**Table 7. Summary of slag and lime assays**

Mass%	CEMEX Slag	Lime-Chemical Lime
Na <sub>2</sub> O	0.34	<0.01
MgO	7.48	0.2
Al <sub>2</sub> O <sub>3</sub>	10.4	0.078
SiO <sub>2</sub>	35.1	0.41
P <sub>2</sub> O <sub>5</sub>	0.02	0.008
S	0.88	0.055
K <sub>2</sub> O	0.27	<0.01
CaO	41.2	74
TiO <sub>2</sub>	0.43	<0.01
Fe <sub>2</sub> O <sub>3</sub>	0.74	0.034
SrO	0.08	0.014
Mn <sub>2</sub> O <sub>3</sub>	0.31	<0.01
LOI	not measured	25
Ca(OH) <sub>2</sub>	not measured	86.4
CaCO <sub>3</sub>	not measured	6.6

## Thermal Analysis

TGA determinations indicated water (1.09%), calcium hydroxide (20.99%), and calcium carbonate (2.92%) gravimetric contents in the sample of lime. The temperature and gravimetric loss profile for this sample is given in Figure 5.



**Figure 5. Thermal curve for the sample of lime (Chemical Lime Co.)**

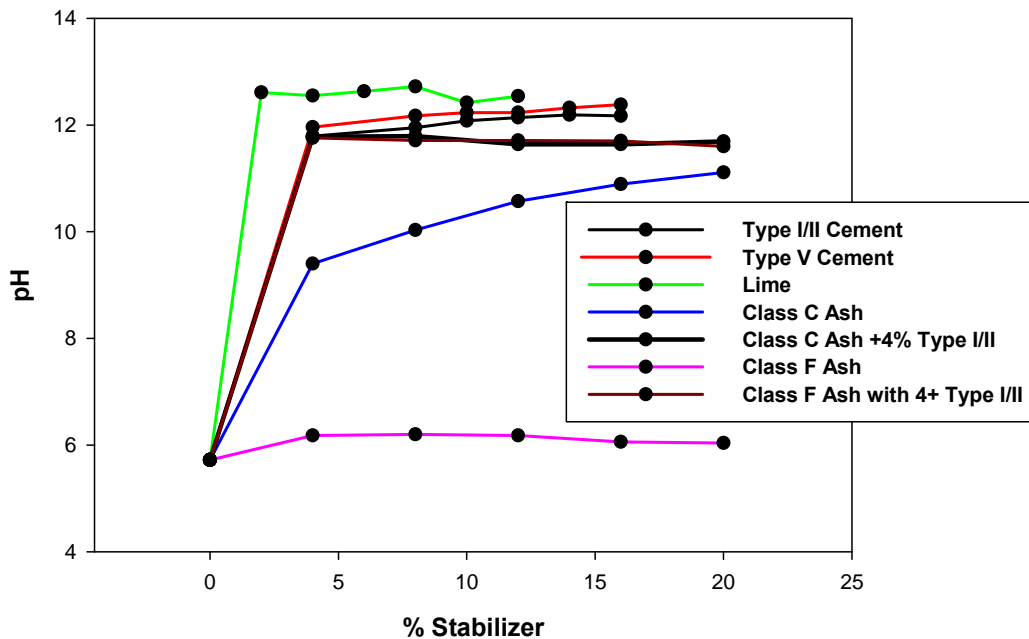
## Soil Stabilization Test Results

To evaluate the effects of the different stabilizers, a total of 47 soil batches were prepared. A summary of batches and properties can be found in Table 8, and the test results are described below. The amount and type of stabilizer was varied between batches, and some batches were treated with a combination of stabilizing materials. Batches are described in Appendix B.



## pH Results

Adding calcium-based stabilizers to soil generally results in a proportional increase in the pH value, up to the maximum pH value as shown in Figure 6. Generally, the maximum pH value can be used as an indicator of the target stabilizer addition rate. Clay minerals can be broken down in a high pH environment and can be irreversibly rendered non-expansive. Most of the stabilizers increased the pH of the soil-stabilizer mixtures, with the exception of the class F fly ash, which caused a slight decrease (< 0.4) in pH values. The greatest increases in pH occurred in lime, cement, and lime/cement blends, with pH values greater than 12. Appendix F contains the pH values of all soils tested.



**Figure 6. S soil pH measurement for various stabilizers**

## Unconfined Compressive Strength

Adding stabilizing materials resulted in a strength increase for most mixtures in the range of 100 to 300 psi, and recommended mixtures are summarized in Figure 7. For comparison, unstabilized soil samples produced unconfined compressive strength of 6 to 37 psi. Strength curves for the tests are presented in Appendix G. Maximum values over 300 psi were obtained in lime and cement/lime blended batches as early as 28 days in some specimens. Limes plus cement produced the highest strengths. Strengths over 200 psi were also measured in materials treated with Type I/II and Type V cement, slag/cement blends, and class F fly ash/cement blends. The values of 7 day 100° F conditioned samples generally predicted the 28 day ambient strength value. Figure 8 shows a sample after breaking, including the stabilizer-soil matrix structure. The area of high visual concentration of the stabilizer indicated how the stabilizer surrounds particles during the mixing process.

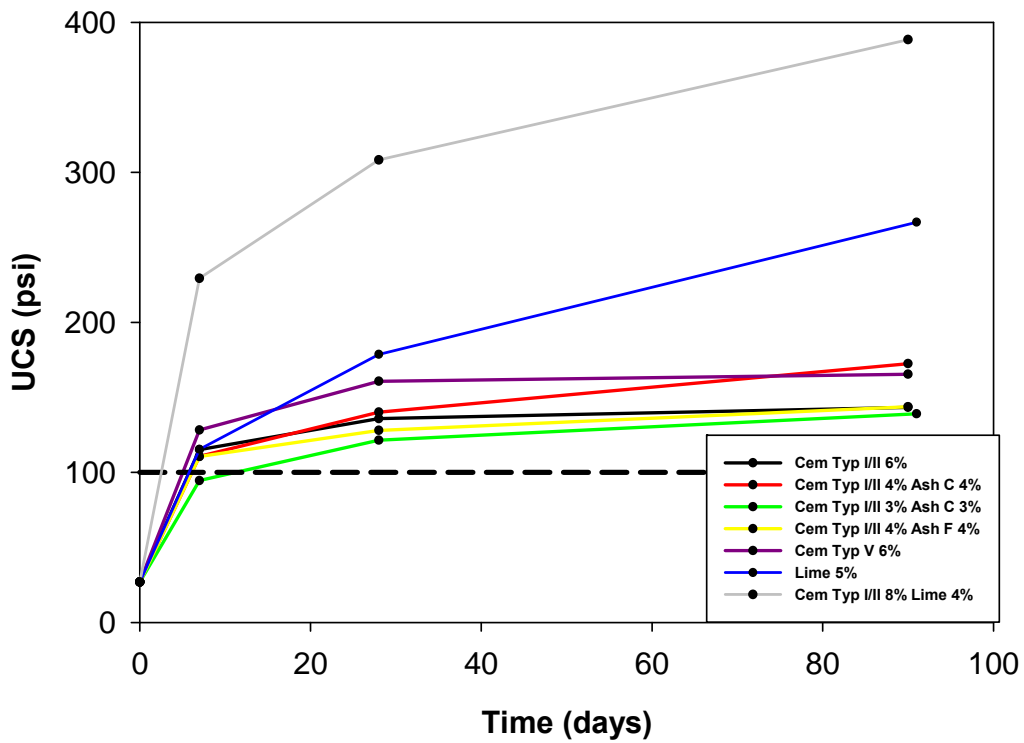
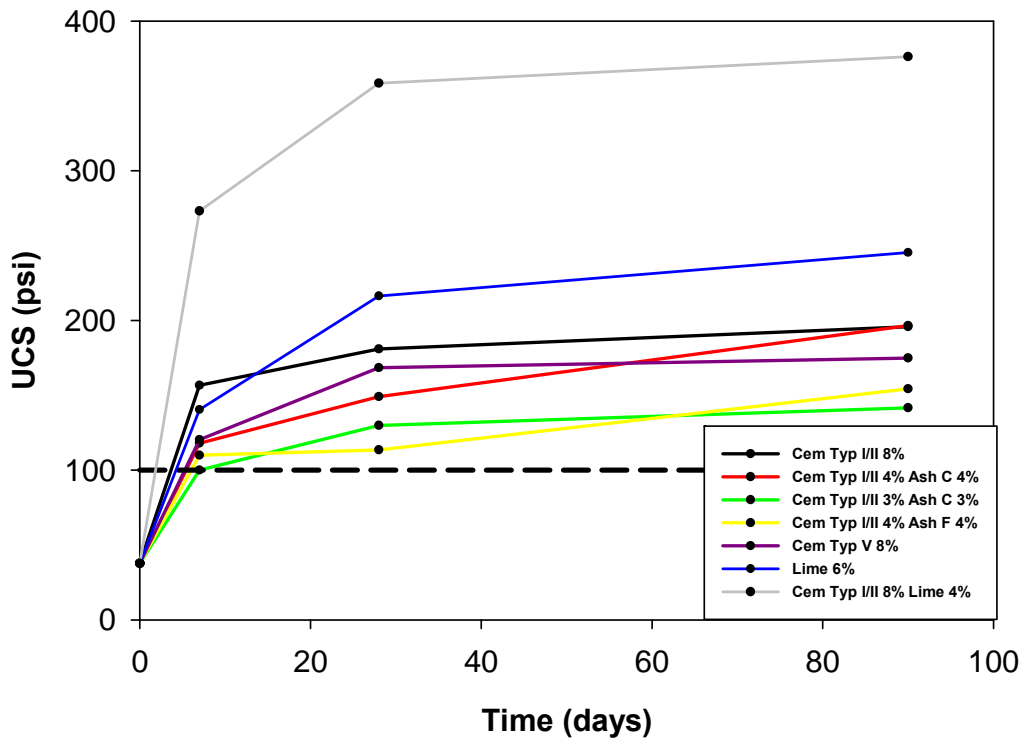
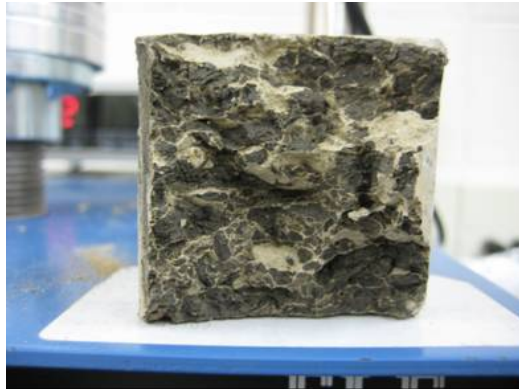


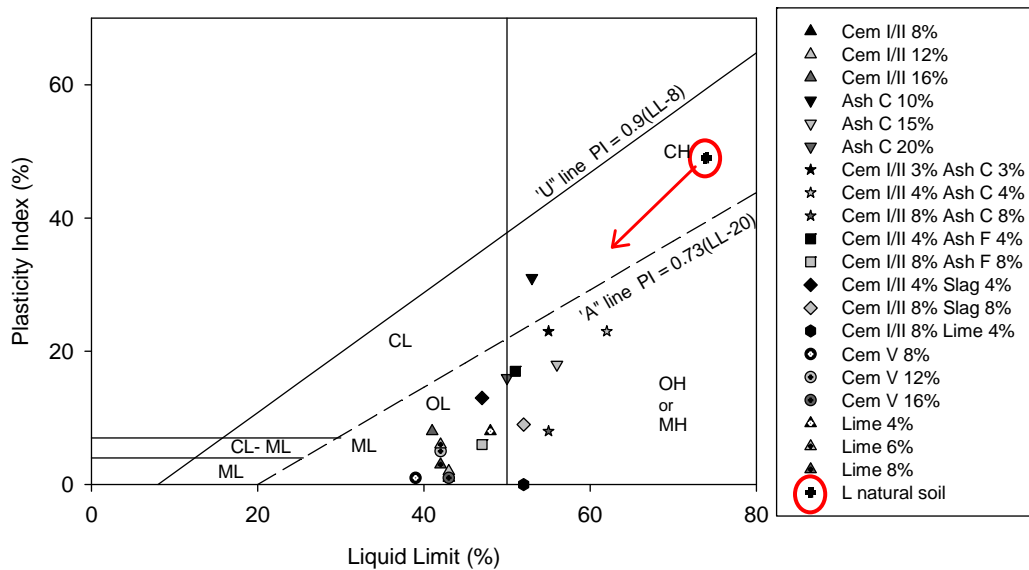
Figure 7. Unconfined compressive strength comparison for L soil (top) and S soil (bottom)



**Figure 8. Soil with class C fly ash failure surface after strength testing**

### Atterberg Limits

Adding stabilizing agents resulted in a decrease of PI and LL in all materials. Figures 9 and 10 show plasticity charts for L and S soils. In these figures, samples are nominal 28+ day ambient temperature curing condition. Type I/II and V cement, lime, and cement/lime blends resulted in the greatest decreases in PI and LL among all the batches. Blended cement/lime stabilized batches resulted in material being rendered non-plastic. Table 9 summarizes the PI and LL values.



**Figure 9. Plasticity chart for soil L batches**

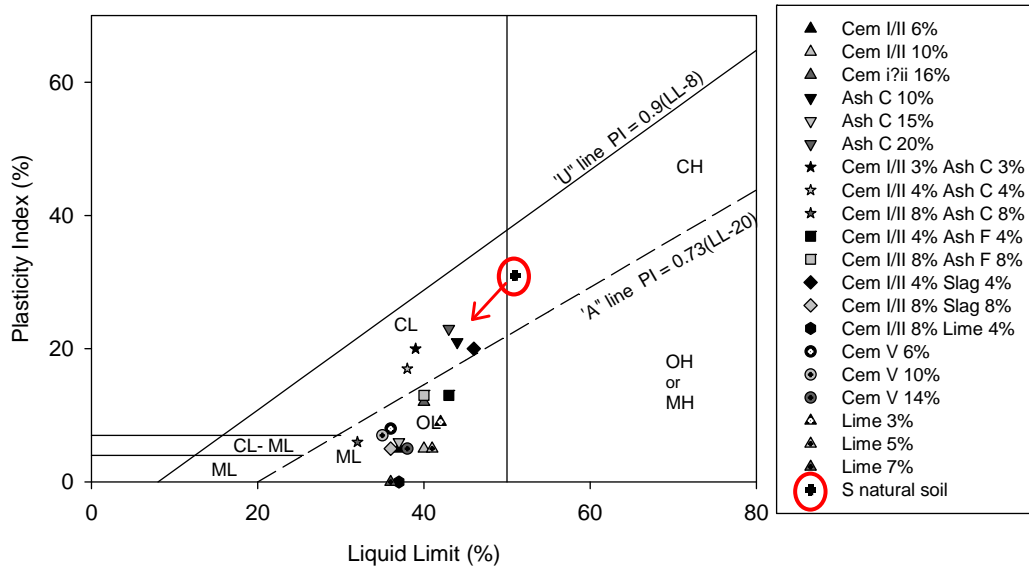


Figure 10. Plasticity chart for soil S batches

Table 9. Summary chart of liquid limit and plastic index of stabilized soils

Sample #	Liquid Limit (Plastic Index)	Sample #	Liquid Limit (Plastic Index)
L Soil Group		S Soil Group	
Natural Soil	74(49)	Natural Soil	51(31)
Cem I/II 8%	43 (1)	Cem I/II 6%	37 (5)
Cem I/II 12%	43 (2)	Cem I/II 10%	40 (5)
Cem I/II 16%	41 (8)	Cem I/II 16%	40 (12)
Ash C 10%	53 (31)	Ash C 10%	44 (21)
Ash C 15%	56 (18)	Ash C 15%	37 (6)
Ash C 20%	50 (16)	Ash C 20%	43 (23)
Cem I/II 8% Ash C 8%	55 (8)	Cem I/II 8% Ash C 8%	32 (6)
Cem I/II 4% Ash C 4%	62 (23)	Cem I/II 4% Ash C 4%	38 (17)
Cem I/II 3% Ash C 3%	55 (23)	Cem I/II 3% Ash C 3%	39 (20)
Cem I/II 8% Ash F 8%	47 (6)	Cem I/II 8% Ash F 8%	40 (13)
Cem I/II 4% Ash F 4%	51 (17)	Cem I/II 4% Ash F 4%	43 (13)
Cem I/II 8% Slag 8%	52 (9)	Cem I/II 8% Slag 8%	36 (5)
Cem I/II 4% Slag 4%	47 (13)	Cem I/II 4% Slag 4%	46 (20)
Cem I/II 8% Lime 4%	52 (NP)	Cem I/II 8% Lime 4%	37 (NP)
Cem V 8%	39 (1)	Cem V 6%	36 (8)
Cem V 12%	42 (5)	Cem V 10%	35 (7)
Cem V 16%	43 (1)	Cem V 14%	38 (5)
Lime 4%	48 (8)	Lime 3%	42 (9)
Lime 6%	42 (6)	Lime 5%	41 (5)
Lime 8%	42 (3)	Lime 7%	36 (NP)

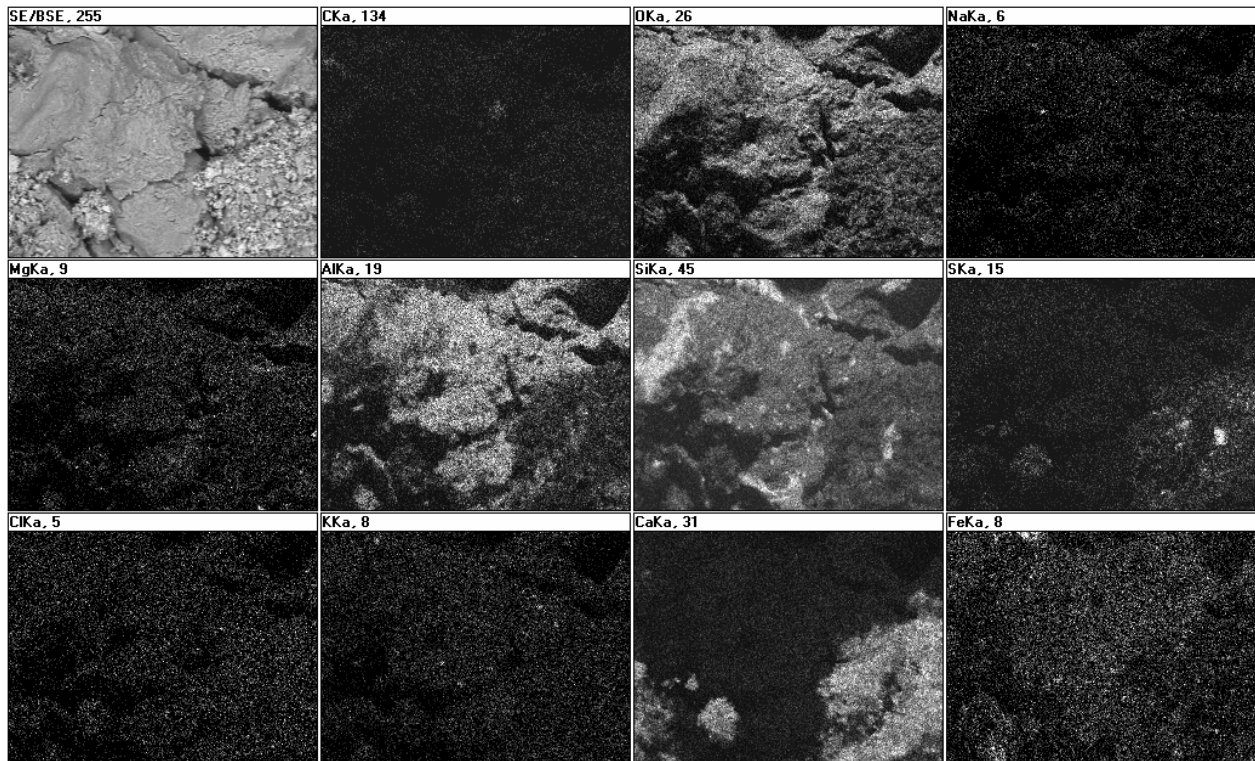
\* highlighted values are recommend mixtures

### *Volumetric Change*

Untreated soil materials had volumetric swell potential percentages that ranged from 12%–20%. Adding stabilizers resulted in a decrease in the swell potential of the soil materials to less than 1%. Class C fly ash did not decrease the swell potential for the S soil. Negative volumetric swell values can be attributed to chemical changes in the stabilizing materials over time, e.g., hydration in cements, and slight variations in laboratory measurements. The high sulfate soil, while showing a decrease from the untreated 20% swell potential, had relatively higher swell values than the lower sulfate materials.

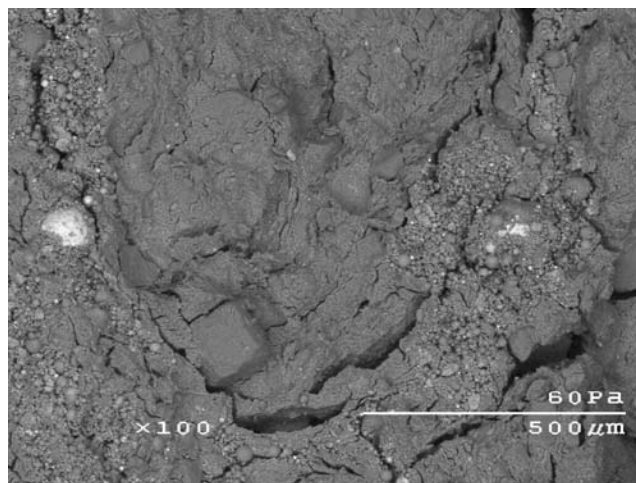
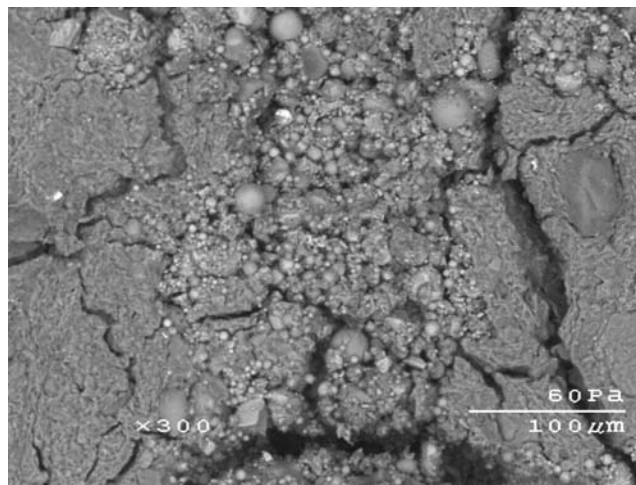
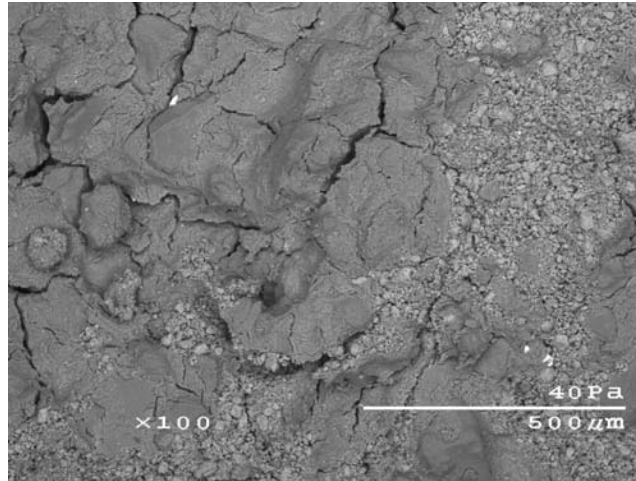
### *SEM Analysis*

SEM examination of the batches did not show the formation of ettringite in samples containing high sulfate contents. A variety of samples were examined, ranging among all soil types and stabilizers. Figures 11 and 12 show SEM images of the recommended mixtures that display some elemental analysis and soil/stabilizer matrix. Additional SEM images and maps can be found in Appendix D.



**Figure 11. EDS map of soil s with type V cement**





**Figure 12.** From top, soil S with type I/II cement, soil L with class C ash and type I/II cement, soil L with class F ash and type I/II cement

## CONCLUSIONS

The preliminary conclusions from the Phase I lab study are as follows:

- Use of portland cement Type I/II and Type V alone and in combination with other stabilizers produces unconfined compressive strengths greater than 100 psi.
- Portland cement in combination with lime produced the highest strength values.
- Use of portland cement Type I/II and Type V alone and in combination with other stabilizers increases the pH values of stabilized soils mixtures.
- Use of portland cement Type I/II and Type V alone and in combination with other stabilizers reduces the PI values of high PI soils.
- Use of portland cement Type I/II and Type V alone and in combination with other stabilizers reduces the volumetric swell potential of soils with expansive properties.

## RECOMMENDATIONS FOR FIELD TRIALS

Based on the results, several material combinations meet the laboratory criteria of 100 psi unconfined compressive strength and less than 2% volume change. The stabilization mixtures proposed for field trials are as follows:

- 6% Cement Type I/II or Type V
- 4% Cement Type I/II + 4% Fly ash Class C
- 4% Cement Type I/II + 4% Fly ash Class F
- 3% Cement Type I/II + 3% Fly ash Class C or F

For each stabilization mixture, three test beds should be constructed that are about 8 ft wide, 150 ft long and 1 ft deep. In addition, six test sections (also 8 ft x 150 ft x 1 ft) should be prepared with no stabilizer to optimize the reclaiming and compaction processes. In total, 18 test beds should be constructed: 12 stabilized test beds and 6 non-stabilized test beds. Table 10 presents a proposed matrix that identifies the details for each test section, each test bed number, and all stabilization quantities. Initial estimates for total material quantities are as follows:

- 35 tons of cement type I/II
- 15 tons of fly ash class C
- 15 tons of fly ash class F
- 12,000 gallons water (depends on natural w% to be verified in the field)

From a research standpoint, alternate test sections could include blends of cement and slag and blends of cement and lime (e.g. 3% Cement Type I/II + 3% Lime).

If the soils selected for the test sections contain high sulfate content, those areas will be considered for a second treatment at a later date. However, the second treatment would depend on field performance monitoring (e.g., evidence of swelling and morphology analysis in the laboratory). After compaction, the test sections should be covered with 2 in. of cover material for curing (well-graded base rock is preferred). For all 18 test beds, a total of about 250 tons of crushed rock would be needed.

Equipment requirements for this work are anticipated to include the following:

- Terex reclaimer (and trained operator) capable of mixing to 12 in. and equipped to inject water through the mixing drum
- Sufficiently large water truck to provide continuous support to the stabilization process
- Suitable mixing water free of soluble sulfates
- Padfoot and smooth drum vibratory rollers with intelligent compaction feedback control and global positioning system (GPS) mapping capability
- Motor grader to pre-rip stabilization areas as needed and to create windrows that contain dry stabilization materials
- Haul truck with spreader bar to distribute the stabilization agents in dry form

As part of this ongoing research effort, the researchers will be onsite for the construction phase to supervise the construction operations and operate the intelligent compaction rollers. The field team, with the ISU Geotechnical Mobile Lab (Figure 13) will help conduct in situ compaction measurements. A tentative schedule for this work is as follows:



**Figure 13. Iowa State University Geotechnical Mobile Laboratory**

- **Day 1.** Mobilize to site and set up intelligent compaction machines and layout test beds.
- **Day 2.** Evaluate mixing efficiency and speed for Terex, conduct compaction study for non-stabilized test strips (6) at target moisture contents and using two different rollers.
- **Day 3.** Distribute, mix, and compact mix test sections 4 and 5. Conduct baseline in situ test measurements.
- **Day 4.** Distribute, mix, and compact mix test sections 2 and 3. Conduct baseline in situ test measurements.
- **Day 5.** Follow up with baseline in situ test measurements.
- **Day 30.** Conduct follow up in situ tests and determine if any retreatment is necessary.
- **Day 90.** Follow up with baseline in situ test measurements.

**Table 10. Summary of proposed field test sections**

Test Section	Test Bed #	Details*	Material Quantities Based on L Soil	Material Quantities Based on S Soil
1	1	Terex reclaimer and high speed. Padfoot roller at high vibration amplitude	<b>No Stabilizer.</b> Anticipate needing to add about to 600 gallons of water to each test section, but depends on initial soil moisture content. Target moisture content is 32%.	<b>No Stabilizer.</b> Anticipate needing to add about 600 gallons of water to each test section, but depends on initial soil moisture content. Target moisture content is 23%.
	2	Terex reclaimer at low speed. Padfoot roller at low vibration amplitude		
	3	Terex reclaimer at optimized speed. Smooth drum roller at high amplitude		
	4	Terex reclaimer at optimized speed. Smooth drum roller at low amplitude		
	5	Terex reclaimer at optimized speed. Padfoot roller at high amplitude for initial 4 passes + 4 passes smooth drum at high amplitude		
	6	Terex reclaimer at optimized speed. Optimized compaction TBD based on roller measurements		
2	7	4% Cement Type I/II + 4% Fly ash Class C. Variable construction operations to optimize process	About <b><u>2.5 tons of Cement + 2.5 tons F.A. + 600 gallons of water</u></b> for each test bed	About <b><u>2.8 tons of Cement + 2.8 tons F.A. + 600 gallons of water</u></b> for each test bed
	8			
	9			
3	10	4% Cement Type I/II + 4% Fly ash Class F. Variable construction operations to optimize process	About <b><u>3.5 tons of Cement + 600 gallons of water</u></b> for each test bed	About <b><u>4.1 tons of Cement + 600 gallons of water</u></b> for each test bed
	11			
	12			
4	13	6% Cement Type I/II or Type V. Variable construction operations to optimize process	About <b><u>2 tons of Cement + 2 tons F.A. + 500 gallons of water</u></b> for each test bed	About <b><u>2.1 tons of Cement + 2.1 tons F.A. + 500 gallons of water</u></b> for each test bed
	14			
	15			
5	16	3% Cement Type I/II + 3% Fly ash Class C or F	About <b><u>2 tons of Cement + 2 tons F.A. + 500 gallons of water</u></b> for each test bed	About <b><u>2.1 tons of Cement + 2.1 tons F.A. + 500 gallons of water</u></b> for each test bed
	17			
	18			

\* Blends of cement and slag and blends of cement and lime (e.g. 3% Cement Type I/II + 3% Lime) could also be included as additional test beds.

### Field Testing

The following testing procedure will be conducted in the ISU Geotechnical Mobile Lab. The purpose is to give baseline data for optimizing the construction process at the field site. These measurements will be performed as soon as possible after arrival on site:

- Standard and modified moisture-density relationship determination
- Atterberg limits determination

- Field determination of sulfate content by colorimetric and conductivity methods

Further onsite lab testing will include

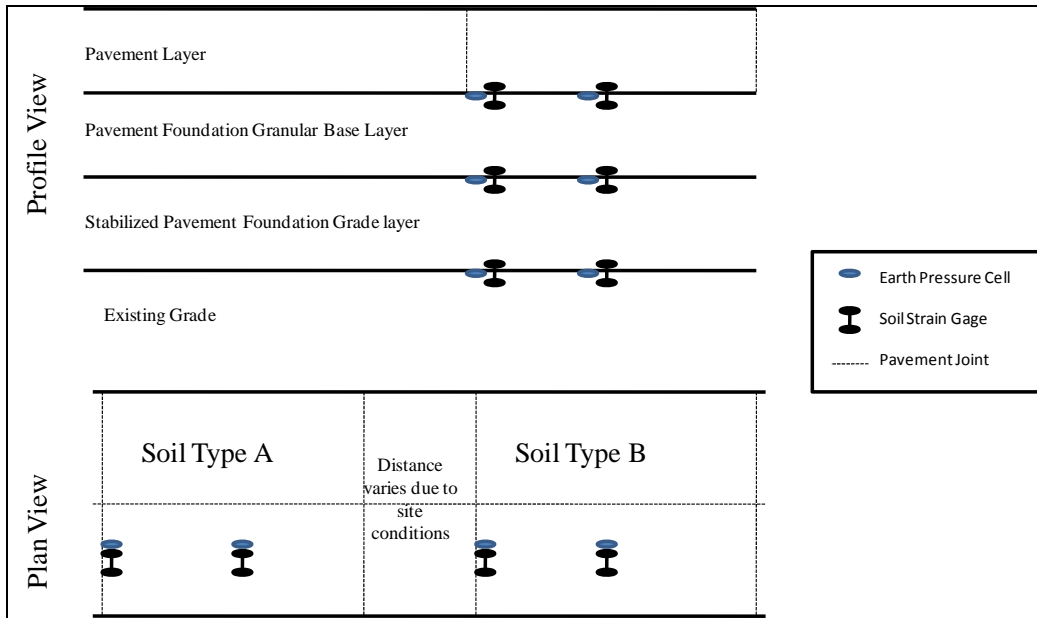
- Particle size analysis
- 2x2 sample preparation of proposed mix proportions of onsite soils and stabilization materials for maturity development
- Resilient modulus samples prepared and tested as maturity develops

Field in situ testing methods will be performed at the time of construction and at follow up testing times. All measurements will be located with real time kinematic GPS to allow for detailed follow up measurements as well as correlation with intelligent compaction (IC) roller measurements. Field in situ testing methods include the following:

- Nuclear density/moisture
- Light Weight deflectometer LWD
- Dynamic cone penetrometer (full depth as needed)
- Plate load tests
- IC roller evaluation

### **Instrumentation for Performance Monitoring**

Installation of in-ground instrumentation would also be beneficial for monitoring the performance and response of stabilized materials. Measurements of in-ground stress and strain will be measured to monitor the long-term volumetric change in the system. Installation, as shown in Figure 14, will allow measurements to be taken in different soil types determined by geological investigations as well as by field investigations to compare soils with differing concentrations of sulfate. Stress and strain measurements at the boundaries of materials may indicate the contribution of different foundation layers to system performance. If the test sections are subsequently paved and jointed pavements are used, measurements at the joints and at mid-panel will indicate variation in the system and the possible effects on pavement faulting. The final number of instrumented locations would depend on the variation in the construction testing area. A minimum of three locations is suggested, with an evaluation period of a minimum of six months.



**Figure 14. Schematic of in-ground instrumentation installation (similar to Pillappa 2005)**

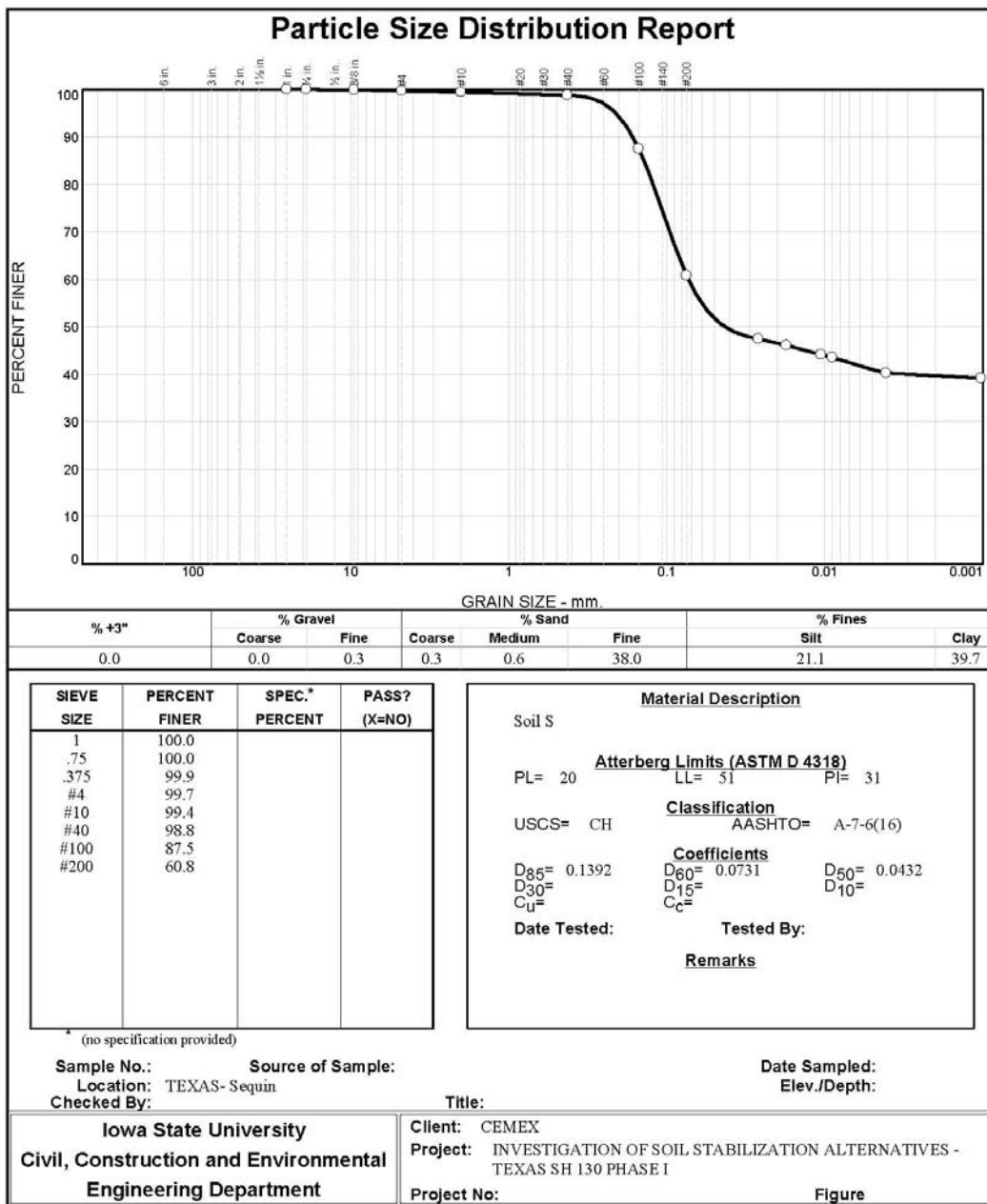
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## APPENDIX A. PARTICLE SIZE ANALYSIS OF S AND L SOILS



Tested By: HG \_\_\_\_\_

**GRAIN SIZE DISTRIBUTION TEST DATA**

9/22/2008

**Client:** CEMEX

**Project:** INVESTIGATION OF SOIL STABILIZATION ALTERNATIVES - TEXAS SH 130 PHASE I

**Location:** TEXAS- Sequin

**Material Description:** Soil S

**PL:** 20

**LL:** 51

**PI:** 31

**USCS Classification:** CH

**AASHTO Classification:** A-7-6(16)

**Sieve Test Data**

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 2920.30  
 Tare Wt. = 0.00  
 Minus #200 from wash = 0.0%

Dry Sample and Tare (grams)	Tare (grams)	Sieve Opening Size	Weight Retained (grams)	Sieve Weight (grams)	Percent Finer
2920.30	0.00	1	0.00	0.00	100.0
		.75	0.00	0.00	100.0
		.375	2.75	0.00	99.9
		#4	5.63	0.00	99.7
72.89	0.00	#10	8.06	0.00	99.4
		#40	0.49	0.00	98.8
		#100	8.27	0.00	87.5
		#200	19.56	0.00	60.8

**Hydrometer Test Data**

Hydrometer test uses material passing #10

Percent passing #10 based upon complete sample = 99.4

Weight of hydrometer sample = 72.86

Hygroscopic moisture correction:

Moist weight and tare = 149.68

Dry weight and tare = 144.57

Tare weight = 49.70

Hygroscopic moisture = 5.4%

Table of composite correction values:

Temp., deg. C: 18.3      21.1      25.9

Comp. corr.: -6.6      -5.5      -4.4

Meniscus correction only = 1.0

Specific gravity of solids = 2.70

Hydrometer type = 152H

Hydrometer effective depth equation:  $L = 16.294964 - 0.164 \times R_m$

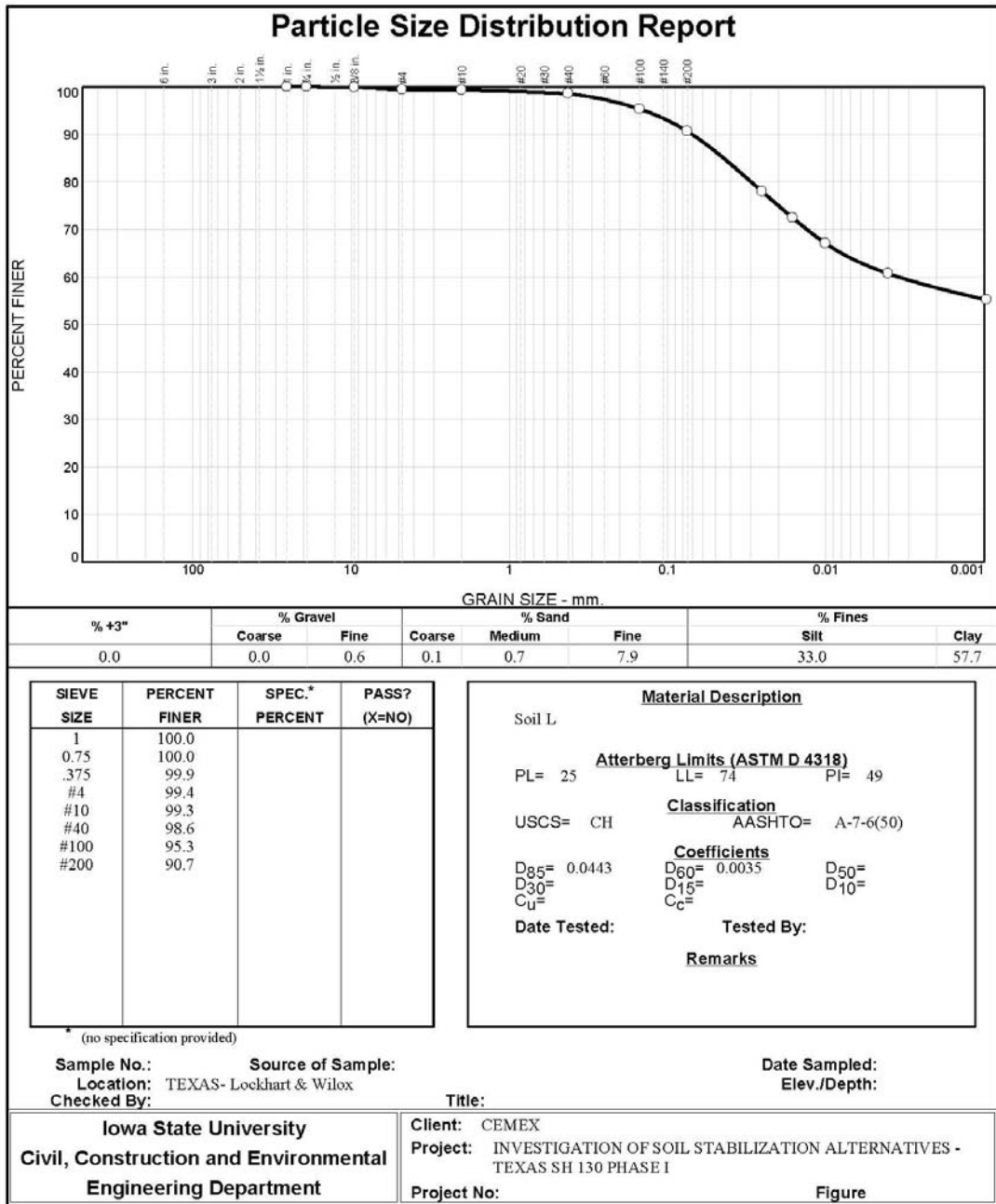
Elapsed Time (min.)	Temp. (deg. C.)	Actual Reading	Corrected Reading	K	Rm	Eff. Depth	Diameter (mm.)	Percent Finer
2.50	20.7	39.0	33.3	0.0133	40.0	9.7	0.0263	47.4
5.75	20.8	38.0	32.4	0.0133	39.0	9.9	0.0175	46.1
16.00	21.1	36.5	31.0	0.0133	37.5	10.1	0.0106	44.1
22.30	21.4	36.0	30.6	0.0132	37.0	10.2	0.0089	43.5
109.00	22.3	33.5	28.3	0.0131	34.5	10.6	0.0041	40.2
1723.00	23.3	32.5	27.5	0.0129	33.5	10.8	0.0010	39.1

**Fractional Components**

Cobbles	Gravel			Sand				Fines		
	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	0.0	0.3	0.3	0.3	0.6	38.0	38.9	21.1	39.7	60.8

D <sub>10</sub>	D <sub>15</sub>	D <sub>20</sub>	D <sub>30</sub>	D <sub>50</sub>	D <sub>60</sub>	D <sub>80</sub>	D <sub>85</sub>	D <sub>90</sub>	D <sub>95</sub>
				0.0432	0.0731	0.1220	0.1392	0.1637	0.2092

Fineness Modulus
0.17



Tested By: HG

**GRAIN SIZE DISTRIBUTION TEST DATA**

9/22/2008

**Client:** CEMEX

**Project:** INVESTIGATION OF SOIL STABILIZATION ALTERNATIVES - TEXAS SH 130 PHASE I

**Location:** TEXAS- Sequin

**Material Description:** Soil S

**PL:** 20

**LL:** 51

**PI:** 31

**USCS Classification:** CH

**AASHTO Classification:** A-7-6(16)

**Sieve Test Data**

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 2920.30  
 Tare Wt. = 0.00  
 Minus #200 from wash = 0.0%

Dry Sample and Tare (grams)	Tare (grams)	Sieve Opening Size	Weight Retained (grams)	Sieve Weight (grams)	Percent Finer
2920.30	0.00	1	0.00	0.00	100.0
		.75	0.00	0.00	100.0
		.375	2.75	0.00	99.9
		#4	5.63	0.00	99.7
		#10	8.06	0.00	99.4
72.89	0.00	#40	0.49	0.00	98.8
		#100	8.27	0.00	87.5
		#200	19.56	0.00	60.8

**Hydrometer Test Data**

Hydrometer test uses material passing #10

Percent passing #10 based upon complete sample = 99.4

Weight of hydrometer sample = 72.86

Hygroscopic moisture correction:

Moist weight and tare = 149.68

Dry weight and tare = 144.57

Tare weight = 49.70

Hygroscopic moisture = 5.4%

Table of composite correction values:

Temp., deg. C: 18.3 21.1 25.9

Comp. corr.: -6.6 -5.5 -4.4

Meniscus correction only = 1.0

Specific gravity of solids = 2.70

Hydrometer type = 152H

Hydrometer effective depth equation:  $L = 16.294964 - 0.164 \times R_m$

Elapsed Time (min.)	Temp. (deg. C.)	Actual Reading	Corrected Reading	K	Rm	Eff. Depth	Diameter (mm.)	Percent Finer
2.50	20.7	39.0	33.3	0.0133	40.0	9.7	0.0263	47.4
5.75	20.8	38.0	32.4	0.0133	39.0	9.9	0.0175	46.1
16.00	21.1	36.5	31.0	0.0133	37.5	10.1	0.0106	44.1
22.30	21.4	36.0	30.6	0.0132	37.0	10.2	0.0089	43.5
109.00	22.3	33.5	28.3	0.0131	34.5	10.6	0.0041	40.2
1723.00	23.3	32.5	27.5	0.0129	33.5	10.8	0.0010	39.1

**Fractional Components**

Cobbles	Gravel			Sand				Fines		
	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	0.0	0.3	0.3	0.3	0.6	38.0	38.9	21.1	39.7	60.8

D <sub>10</sub>	D <sub>15</sub>	D <sub>20</sub>	D <sub>30</sub>	D <sub>50</sub>	D <sub>60</sub>	D <sub>80</sub>	D <sub>85</sub>	D <sub>90</sub>	D <sub>95</sub>
				0.0432	0.0731	0.1220	0.1392	0.1637	0.2092

<b>Fineness Modulus</b>
0.17









Prep Date 5/16/2008

Description L-CI II-FC-32-3\_4 3746g of Soil L @ 32% with 107.9g of Type I II(3%) and 107.9g of type C Fly Ash(3%)

Table with columns: Condition Samples, As mixed, Weight (g), Height (in), Dia. (in), Strength(lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, and 90 Day Ambient.

Prep Date 2/24/2008 12:55 PM

Description L-CI II-FF-32-8\_8 Soil L @32% with 8% Type I II Cement and 8% Class F Fly Ash

Table with columns: Condition Samples, As mixed, Weight (g), Height (in), Dia. (in), Strength(lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, and 93 Day Ambient.

Prep Date 4/24/2008

Description L-CI II-FF-32-4\_4 5045 g L @32% with 152.9g Type I II and 152.9g Class F Ash

Table with columns: Condition Samples, As mixed, Weight (g), Height (in), Dia. (in), Strength(lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 28 Day Ambient, and 90 Day Ambient.

Prep Date 2/23/2008 12:35 pm

Description L-CI II-S-36-8\_8 Soil L @ 36% with 8% Type I II cement and:

Table with columns: Condition Samples, As mixed (Weight, Height), 7 Day 100 F (Weight, Height, Dia, Strength, Avg), Date (3/1/2008), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes 3 Day Soaked and 28 Day Ambient tests.

Prep Date 5/9/2008

Description L-CI II-S-32-4\_4 3863.6g dry Soil L @ 32% with 154.5g Type III(4%) and 154.5g Slag(4%)

Table with columns: Condition Samples, As mixed (Weight, Height), 7 Day 100 F (Weight, Height, Dia, Strength, Avg), Date (16-May-08), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes 3 Day Soaked and 28 Day Ambient tests.

Prep Date 2/24/2008

Description L-CI II-L-36-8\_4 Soil L @ 36% with 8% Type I II cement and 4% Lime

Table with columns: Condition Samples, As mixed (Weight, Height), 7 Day 100 F (Weight, Height, Dia, Strength, Avg), Date (7-Aug-08), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes 3 Day Soaked and 28 Day Ambient tests.



Prep Date 4/23/2008

Description

L-L-33-4 5291g L@ 33% with 159g Lime(4%)

Condition

Samples

Table with columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Rows include 'As mixed', '7 Day 100 F', '7 Day Ambient', '28 Day Ambient', and '90 Day Ambient' with '3 Day Soaked' annotations.

Prep Date 5/9/2008

Description

L-L-33-6 5000g Soil L @ 33% with 227.3g Lime(6%) add 1% water

Condition

Samples

Table with columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Rows include 'As mixed', '7 Day 100 F', '7 Day Ambient', '28 Day Ambient', and '90 Day Ambient' with '3 Day Soaked' annotations.

Prep Date 5/9/2008

Description L-L-34-8 5000g of Soil L @ 34% with 303.12g Lime(8%) add

2% water(76g)

Condition

Samples

Table with columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Rows include 'As mixed', '7 Day 100 F', '7 Day Ambient', '28 Day Ambient', and '90 Day Ambient' with '3 Day Soaked' annotations.



Prep Date 3/3/2008 8:00 AM

Description S-CI II-25-16 5289g Soil S @ 25% with 16% Type II Cement (677g)

Table with 15 columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 28 Day Ambient, and 91 Day Ambient.

Prep Date 3/3/2008 10:15am

Description S-FC-23-10 5200g Soil S @23% with 10% Class C Fly ash (423g)

Table with 15 columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 28 Day Ambient, and 91 Day Ambient.

Prep Date 3/3/2008 1:00 PM

Description S-FC-23-15 5200g Soil S @23% with 15% Class C Fly Ash (634.7g)

Table with 15 columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 28 Day Ambient, and 91 Day Ambient.





Prep Date 5/16/2008

Description S-CI II-FC-23-3\_34970g of Soil S @ 23% with 121.21g of Type I III(3%) and 121.21g of type C Fly Ash(3%)

Table with columns: Condition Samples, As mixed, Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-headers for 7 Day 100 F, 7 Day Ambient, and 28 Day Ambient.

Prep Date 3/6/2008 10:00 AM

Description S-CI II-FF-25-8\_8.5100g Soil S 225% with 326.4g Type I III(8%) and 326g Class F Fly Ash(8%)

Table with columns: Condition Samples, As mixed, Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-headers for 7 Day 100 F, 7 Day Ambient, and 28 Day Ambient.

Prep Date 5/8/2008

Description S-CI II-FF-23-4\_4.5000g of Soil S @ 23% with 162.6g Type I III(4%) and 162.6g Class F Fly Ash(4%)

Table with columns: Condition Samples, As mixed, Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-headers for 7 Day 100 F, 7 Day Ambient, and 28 Day Ambient.

Prep Date 3/6/2008 11:45am

Description S-CI-II-S-25-8\_8 5100g Soil S225% with 326.4g Type I II(8%) and 326.4g Slag(8%)

Table with 15 columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 31 Day Ambient, and 91 Day Ambient, with 'Soaked' and 'Date' labels.

Prep Date 3/6/2008

Description S-CI II-L-25-8\_4 5200g Soil S @25% with 332.8 g Type I II(8%) and 166.4g Lime (4%)

Table with 15 columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 31 Day Ambient, and 91 Day Ambient, with 'Soaked' and 'Date' labels.

Prep Date 3/6/2008 4:00 PM

Description S-L-23-3 5174g Soil S @23% with 126.2g Lime(3%)

Table with 15 columns: Weight (g), Height (in), Dia. (in), Strength (lbs), Strength (psi), Avg (psi), Swell Ht, Swell Dia, Vol Swell, Δ Weight, % H2O, Dry Dens, Avg Vol Swell, Avg Dry Dens. Includes sub-sections for 7 Day 100 F, 7 Day Ambient, 29 Day Ambient, and 91 Day Ambient, with 'Soaked' and 'Date' labels.





Prep Date 2/19/2008

Description

Soil S with no stabilizer

Condition As mixed

Samples	As mixed		Initial				Date						
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
21%	205.16	2.205	250.9	2.503	2.187	4	0.8	13.51%	8.59%	33.85%	22.29%	47.8%	68.65
24.00%	200.31	2.079	228.47	2.261	2.116	8	2.0	8.75%	5.48%	21.01%	14.06%	41.5%	73.92
27.00%	198.57	1.904	226.22	2.171	2.159	8	2.2	14.02%	7.63%	32.08%	13.92%	43.8%	73.22
30.00%	205.59	2.027	217.58	2.151	2.067	23	6.3	6.12%	3.04%	12.67%	5.83%	37.3%	83.46
33.00%	205.88	2.08	219.44	2.169	2.064	12	3.2	4.28%	2.89%	10.40%	6.59%	41.5%	83.72

Prep Date 4/1/2008

Description

?-Cl II-34-10 Soil 750g at 34% WITH 56G Type I II Cement

Condition As mixed

Samples	As mixed		7 Day Ambient				Date						
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	198.01	1.982	197.93	1.99	2.006	314	101	0.40%	0.05%	0.50%	-0.04%	29.4%	88.1
2	197.99	1.978	207.27	2.049	2.04	108	33	3.59%	1.75%	7.24%	4.69%	35.7%	86.7
3	197.99	1.978	208.52	2.054	2.035	141	43	3.84%	1.50%	6.97%	5.32%	34.1%	88.5
4	199.84	1.993	205.2	2.035	2.037	205	63.0	2.11%	1.60%	5.39%	2.68%	31.5%	87.7

Prep Date 4/1/2008

Description

?-L-34-5750g ? @ 34% with 28g Lime

Condition As mixed

Samples	As mixed		7 Day Ambient				Date						
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	197.94	2.004	197.82	2.038	2.032	309	95	1.70%	1.35%	4.45%	-0.06%	32.1%	81.5
2	196.73	1.995	209.01	2.119	2.075	123	35	6.22%	3.49%	13.76%	6.24%	39.5%	79.5
3	197.26	2.002	206.72	2.12	2.077	134	38	5.89%	3.59%	13.64%	4.80%	38.3%	79.1
4	176.74	1.785	180.58	1.854	2.055	222	82.2	3.87%	2.49%	9.11%	2.17%	33.6%	80.7

Prep Date 4/1/2008

Description

?-L-Cl II-34-4 8 750g ? @ 34% with 44.8g Type I II and 22.4 g Lime

Condition As mixed

Samples	As mixed		7 Day Ambient				Date						
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	198.32	1.993	198.23	2.004	2.012	439	139.2	0.55%	0.35%	1.26%	-0.05%	29.6%	91.3
2	197.6	1.996	210.05	2.082	2.054	184	54.0	4.31%	2.44%	9.47%	6.30%	36.7%	84.6
3	196.38	1.983	209.61	2.095	2.063	232	67.3	5.65%	2.89%	11.85%	6.74%	35.7%	83.9
4	199.1	2.022	213.43	2.135	2.085	259	72.3	5.59%	3.99%	14.18%	7.20%	35.8%	82.0

Prep Date 4/1/2008

Description

?-FC-34-15 750g ? Soil at 34% with 84 g Class C Ash

Condition As mixed

Samples	As mixed		7 Day Ambient					Date					
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	197.13	1.954	197.04	1.95	2.006	198	66.3	-0.20%	0.05%	-0.11%	-0.05%	27.5%	92.3
2	198.3	1.964	204.95	2.01	2.019	61	19.2	2.34%	0.70%	3.78%	3.35%	31.7%	91.9
3 Day Soaked			28 Day Soaked					Date 5/4/2008					
3	200.48	2.001	204.55	2.015	2.015	94	29.5	0.70%	0.50%	1.71%	2.03%	32.0%	91.7
3 Day Soaked			90 Day Soaked					Date					
4	201.49	2.006	203.1	2.004	2.015	163	51.7	-0.10%	0.50%	0.90%	0.80%	29.8%	91.5

Prep Date 4/8/2005

Description ?-S-CI II-34-8 8750g ? Soil @ 34% with 44.8g Type I II and 44.8 g Slag

Condition As mixed

Samples	As mixed		7 Day Ambient					Date					
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	198.55	2.003	198.48	2.028	2.01	383	118.6	1.25%	0.25%	1.75%	-0.04%	27.2%	92.2
2	198.78	1.995	209.8	2.072	2.031	162	48.0	3.86%	1.30%	6.57%	5.54%	32.6%	89.6
3 Day Soaked			28 Day Soaked					Date					
3	198.41	1.991	205.67	2.032	2.025	182	56.1	2.06%	1.00%	4.11%	3.66%	32.0%	90.5
3 Day Soaked			90 Day Soaked					Date					
4	198.45	1.985	203.15	2.008	2.023	203	64.1	1.16%	0.90%	2.98%	2.37%	31.0%	90.6

Prep Date 4/8/2008

Description ?-FC-CI II-34-8 8750g ? @ 34% with 44.8g Class C Ash and 44.8g Type I II Cement

Condition As mixed

Samples	As mixed		7 Day Ambient					Date					
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	198.13	1.975	198.03	1.978	2.009	387	126	0.15%	0.20%	0.55%	-0.05%	26.6%	90.5
2	199.94	1.996	208.2	2.09	2.044	125	36	4.71%	1.95%	8.82%	4.13%	32.6%	87.0
3 Day Soaked			28 Day Soaked					Date 5/9/2008					
3	200.38	2.001	207.53	2.049	2.037	128	38.8	2.40%	1.60%	5.69%	3.57%	32.5%	89.2
3 Day Soaked			90 Day Soaked					Date					
4	198.94	1.989	204.92	2.019	2.031	162	50.6	1.51%	1.30%	4.16%	3.01%	30.0%	89.9

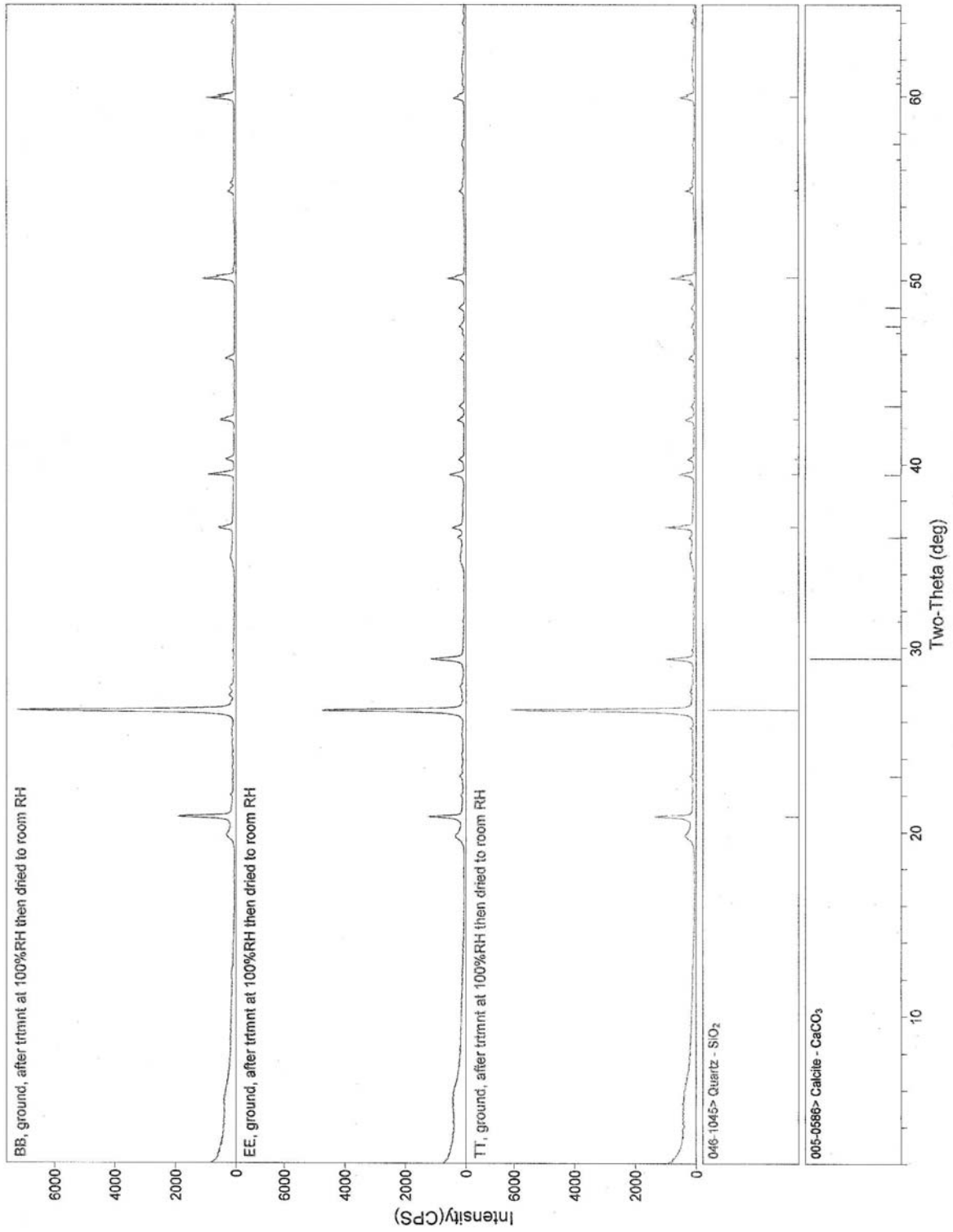
Prep Date 4/8/2008

Description ?-FF-CI II- 34-8 8750g ? @ 34% with 44.8g Type I II and 44.8g

Condition As mixed

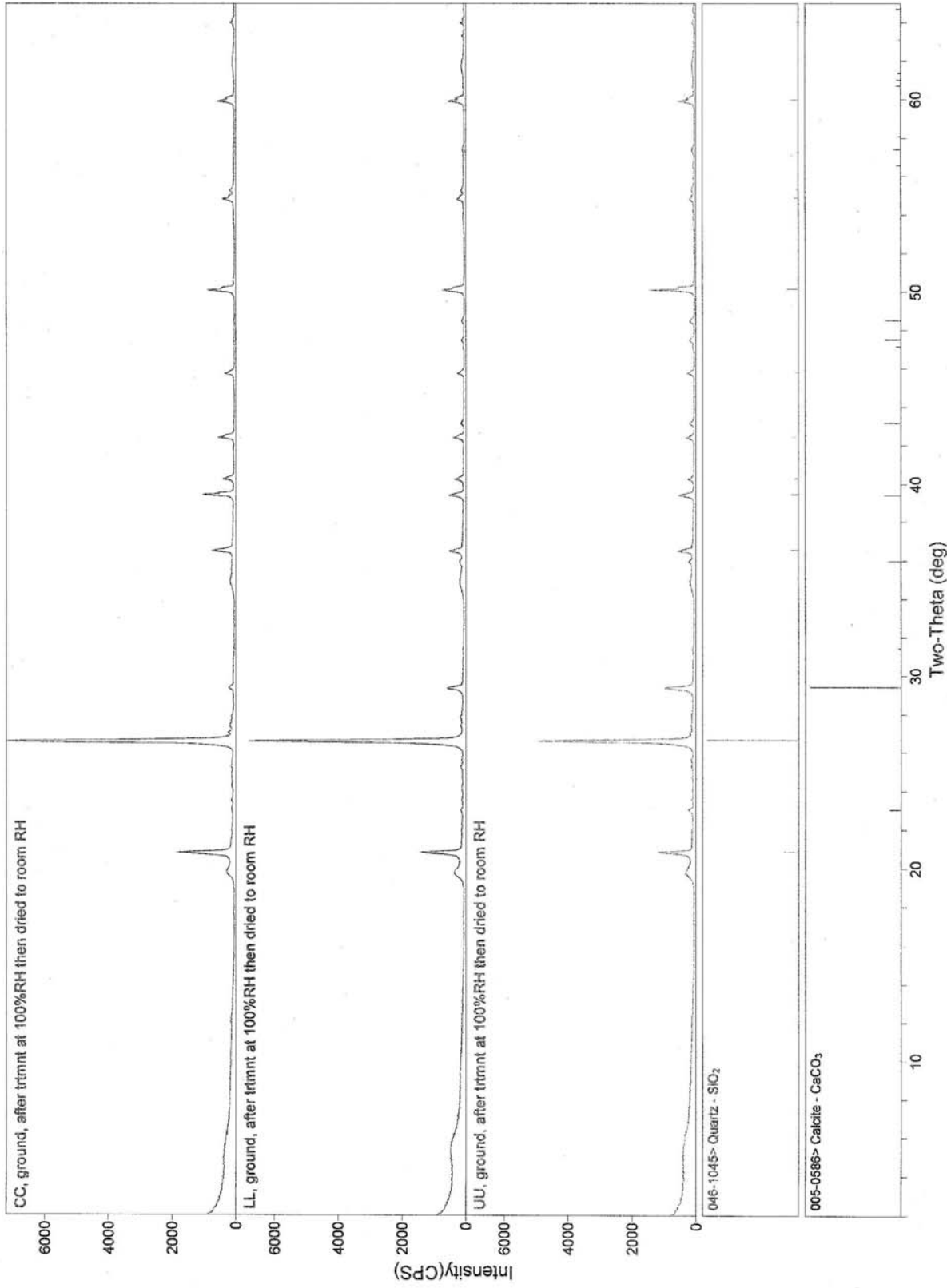
Samples	As mixed		7 Day Ambient					Date					
	Weight (g)	Height (in)	Weight (g)	Height (in)	Dia. (in)	Strength(lbs)	Strength (psi)	Swell Ht	Swell Dia	Vol Swell	Δ Weight	% H2O	Dry Dens
1	194.58	1.978	194.32	1.978	2.009	276	89.8	0.00%	0.20%	0.40%	-0.13%	28.0%	92.0
2	197.8	1.987	205.74	2.029	2.033	122	37.7	2.11%	1.40%	4.99%	4.01%	33.1%	89.2
3 Day Soaked			28 Day Soaked					Date 5/9/2008					
3	198.89	2.01	208	2.065	2.038	130	38.8	2.74%	1.65%	6.15%	4.58%	33.3%	88.0
3 Day Soaked			90 Day Soaked					Date					
4	199.08	2.01	203.57	2.038	2.02	189	57.9	1.39%	0.75%	2.92%	2.26%	30.1%	88.9

# APPENDIX C. SOIL XRD DIFFRACTOGRAMS

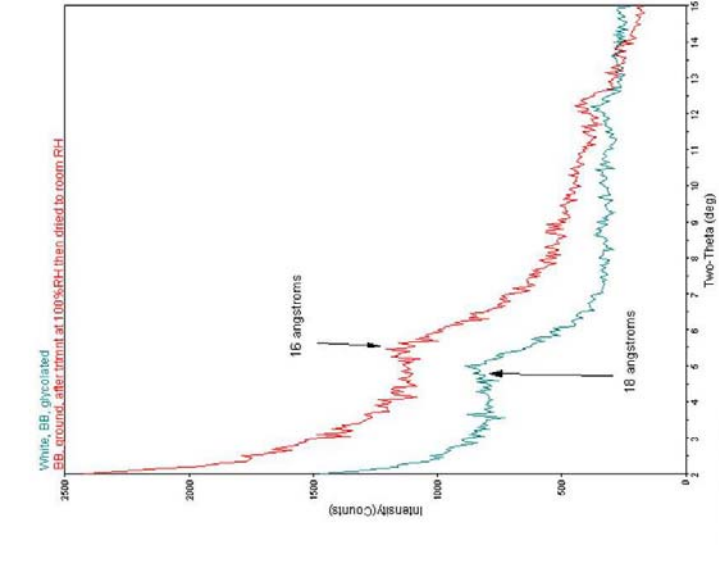
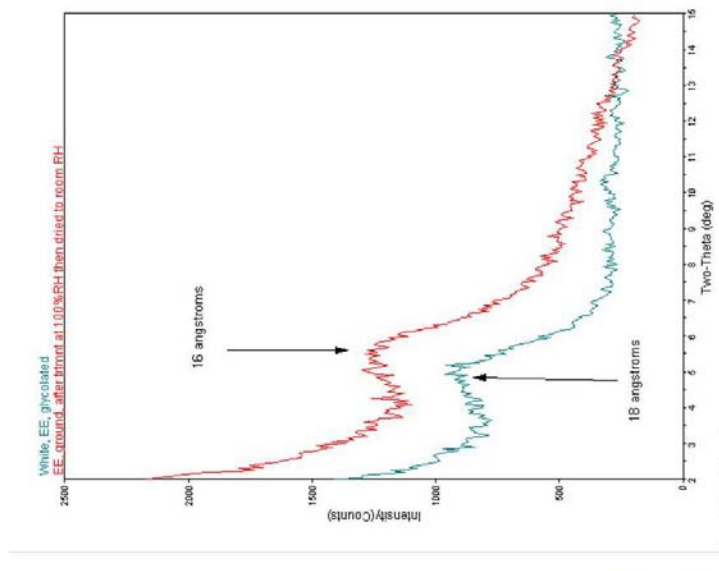
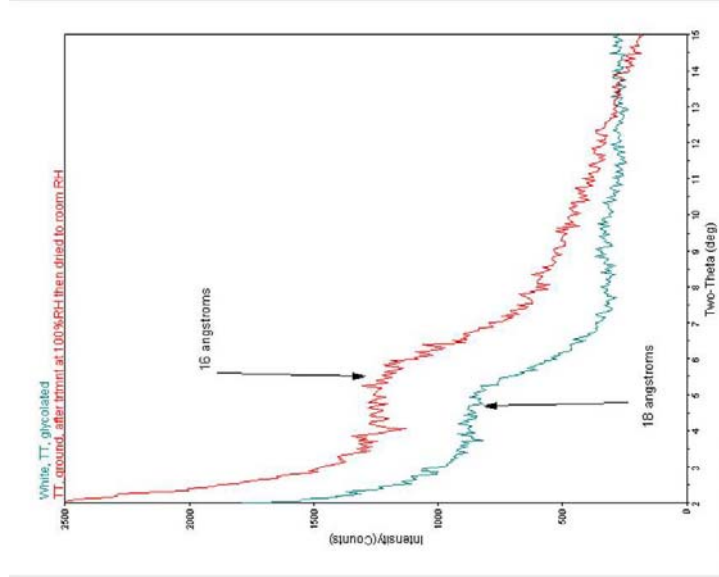


**Diffractograms of received soils, BB is S soil type, EE and TT are L soil Type**

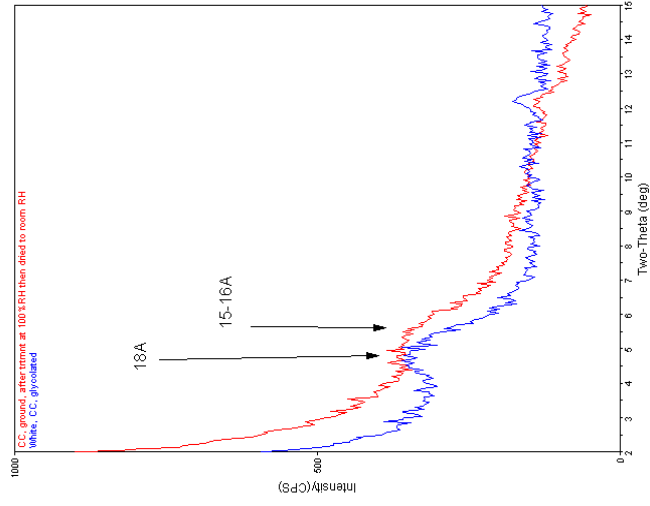
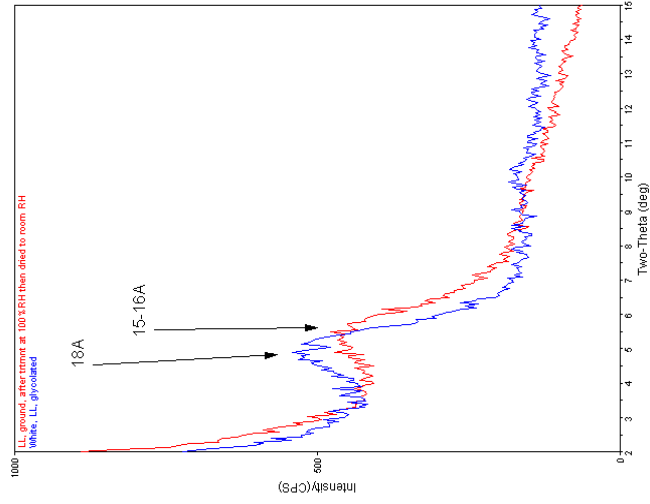
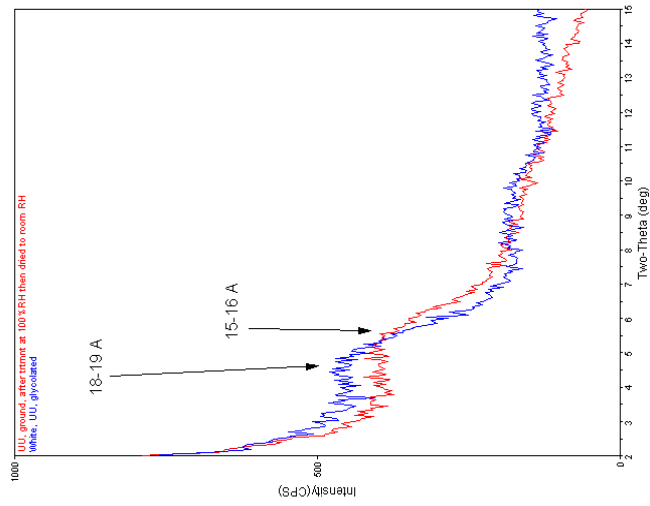




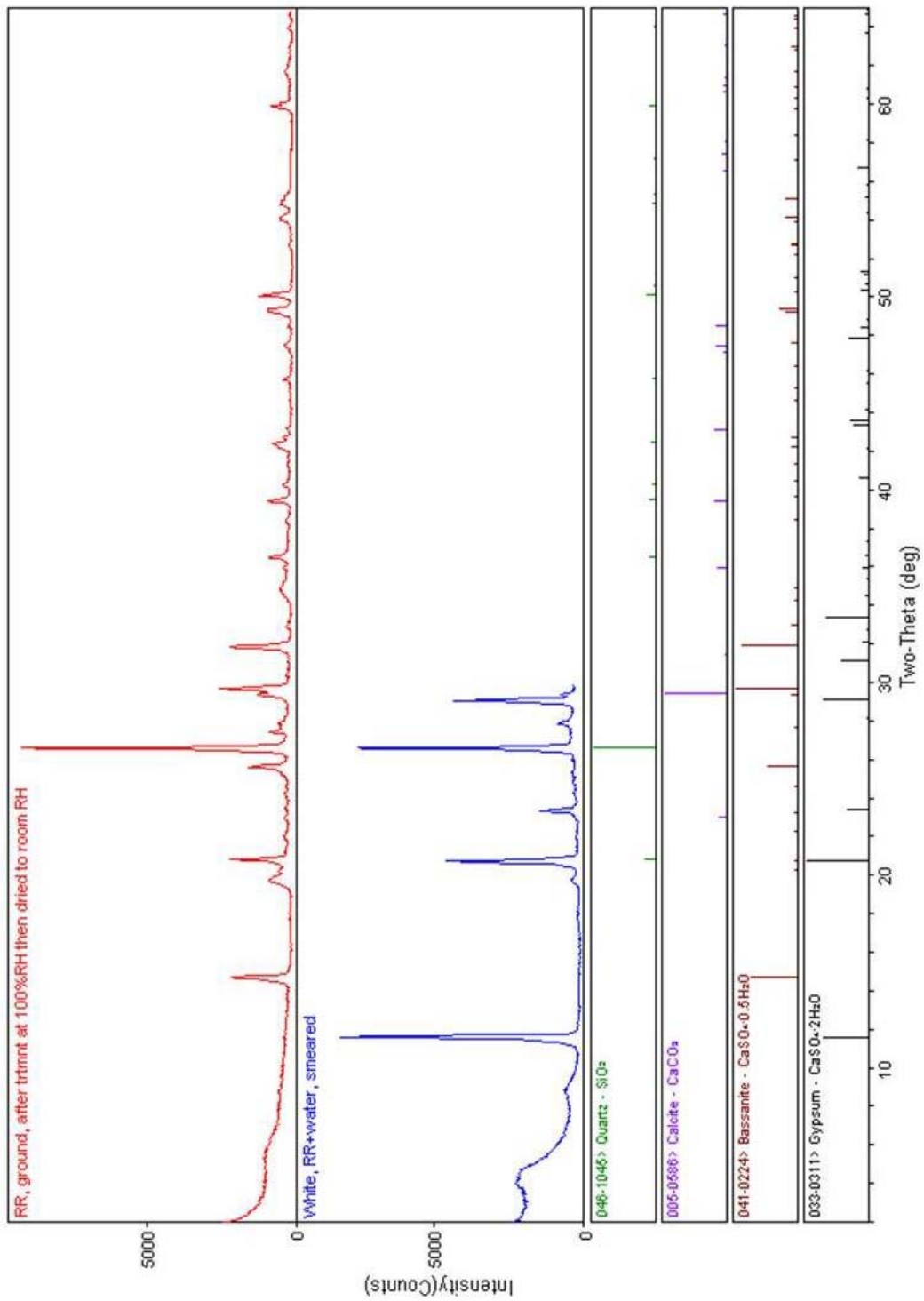
**Diffractograms of received soils, CC is S soil type, LL and UU are L soil Type**



**Diffractograms of received soils, BB is S soil type, EE and TT are L soil type. Overlays show glycolated treatment for expansive mineral investigation. Samples display presence of swelling clays.**



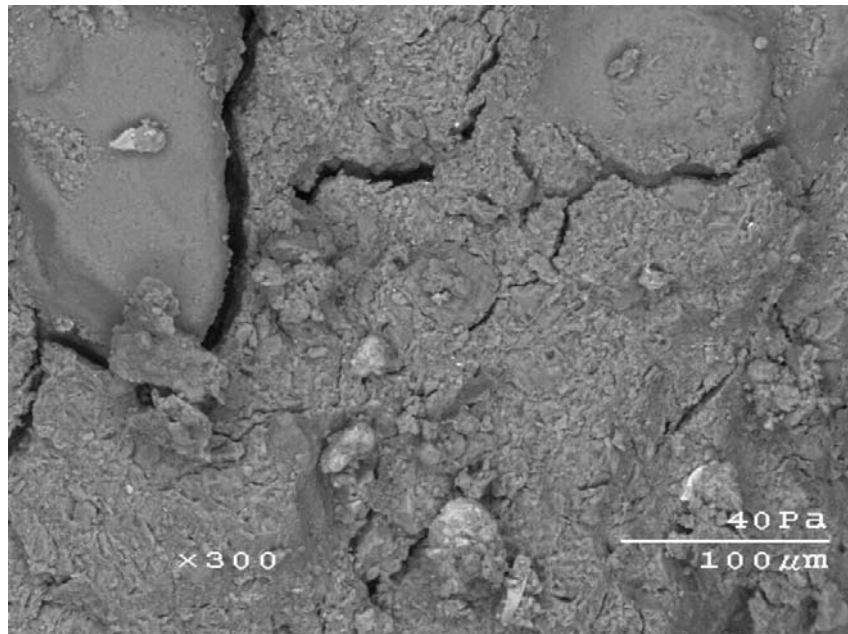
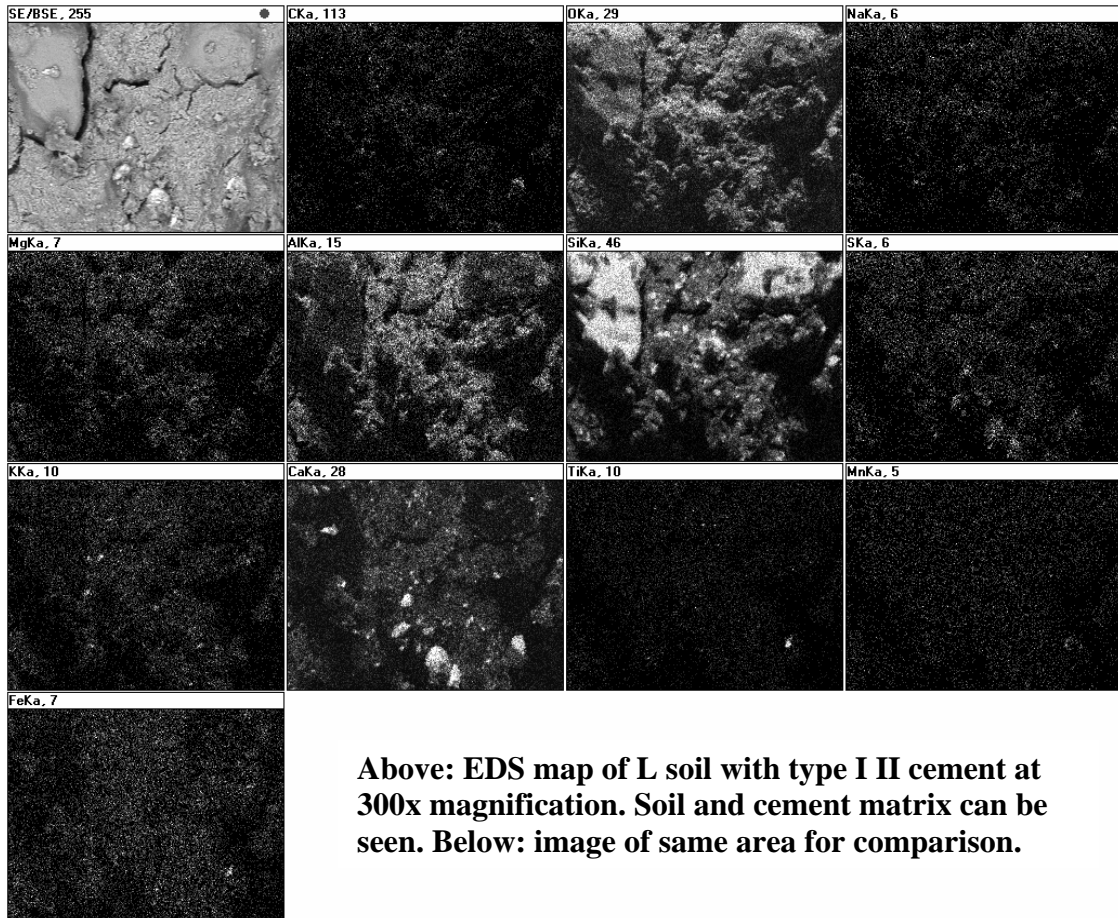
**Diffractograms of received soils, CC is S soil type, LL and UU are L soil type. Overlays show glycolated treatment for expansive mineral investigation. Samples display presence of swelling clays**

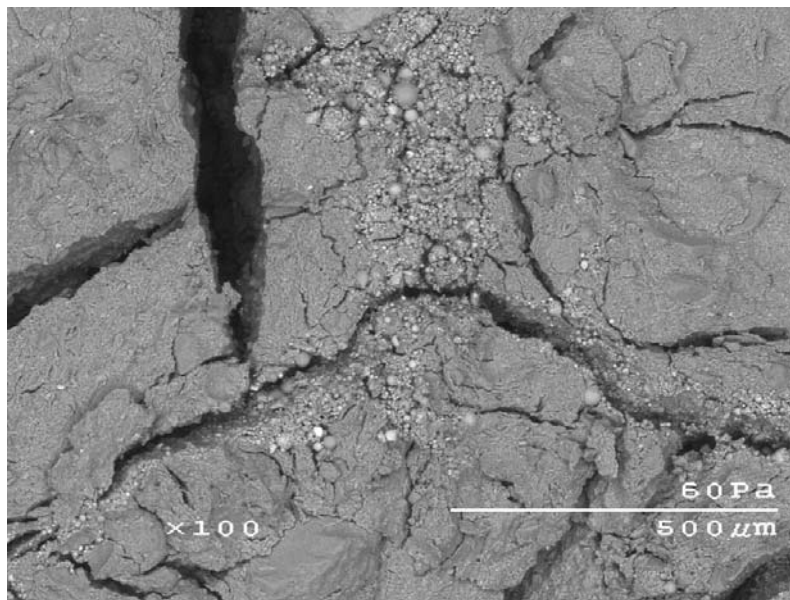
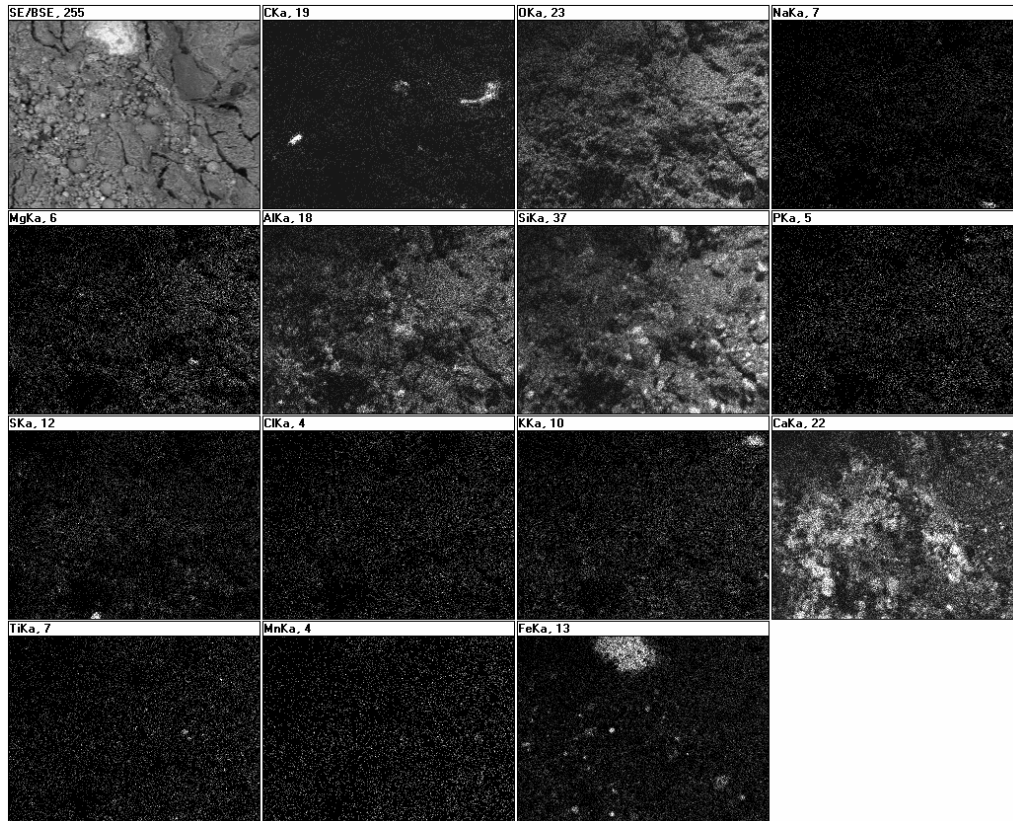


**Diffractograms of RR soil containing high sulfate content. Samples treated at 100% RH and water treatment. Water treated sample was smeared onto slide causing some orientation of clay minerals. Water treatment resulted in hydration of bassanite to gypsum and expansion of clay basal plane to 20 angstroms.**

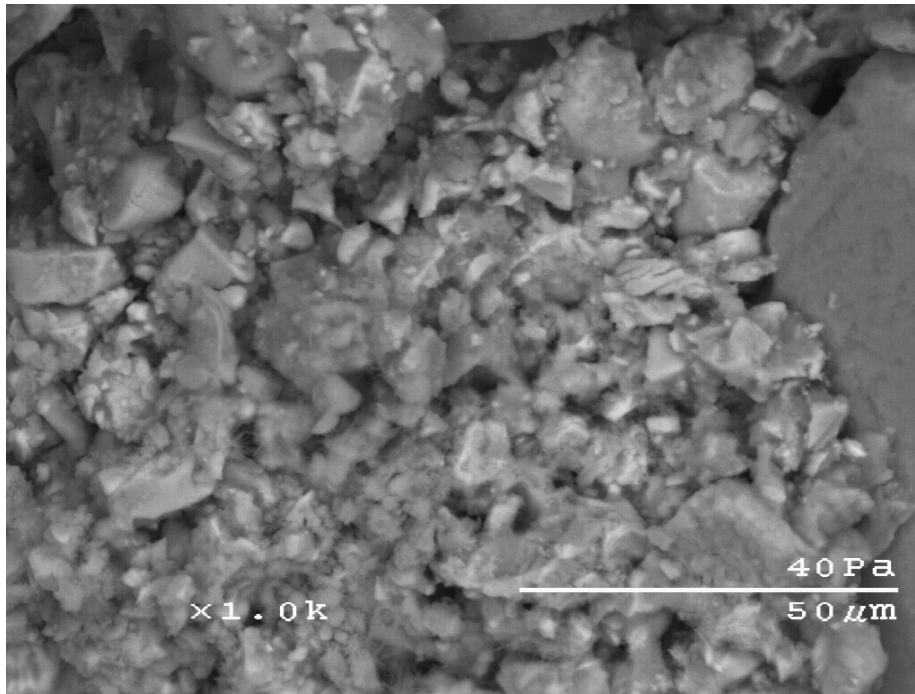


## APPENDIX D. SEM IMAGES

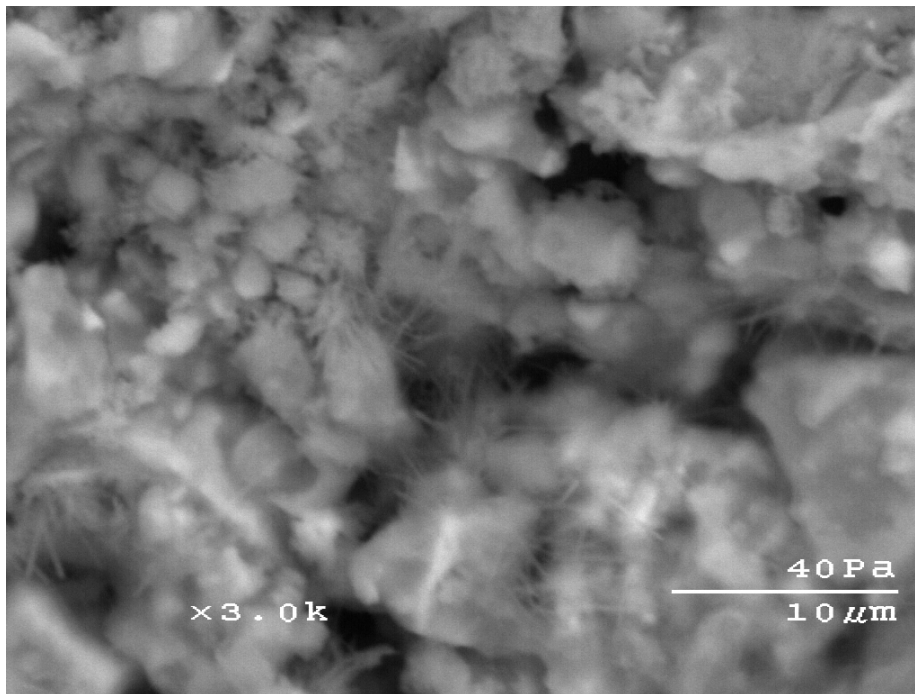




**Top: EDS map of L soil with type I II cement and class F fly ash at 300x magnification.  
 Bottom: L with type I II cement and class C fly ash at 100x magnification. Soil and cement matrix can be seen.**

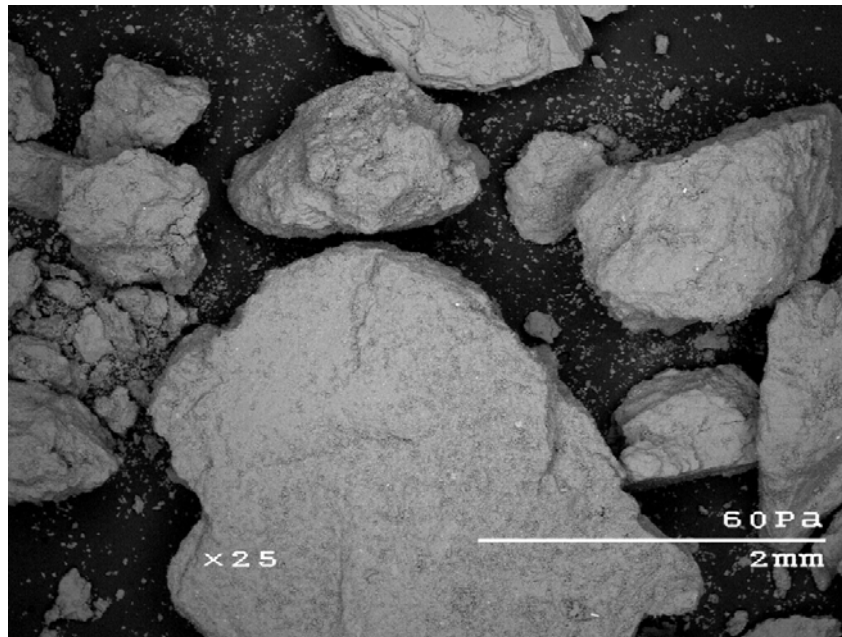
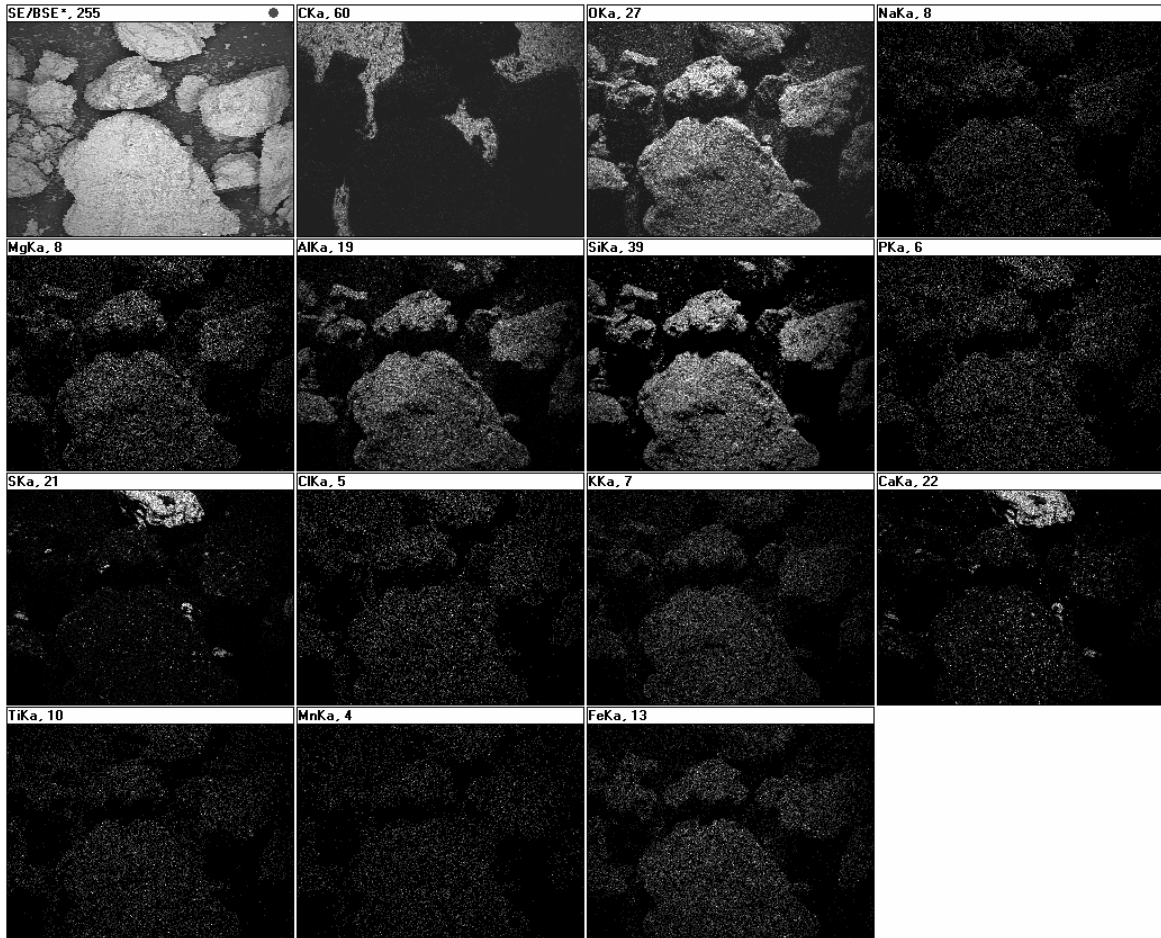


**Soil L with lime and type I II cement, magnified 1000x**

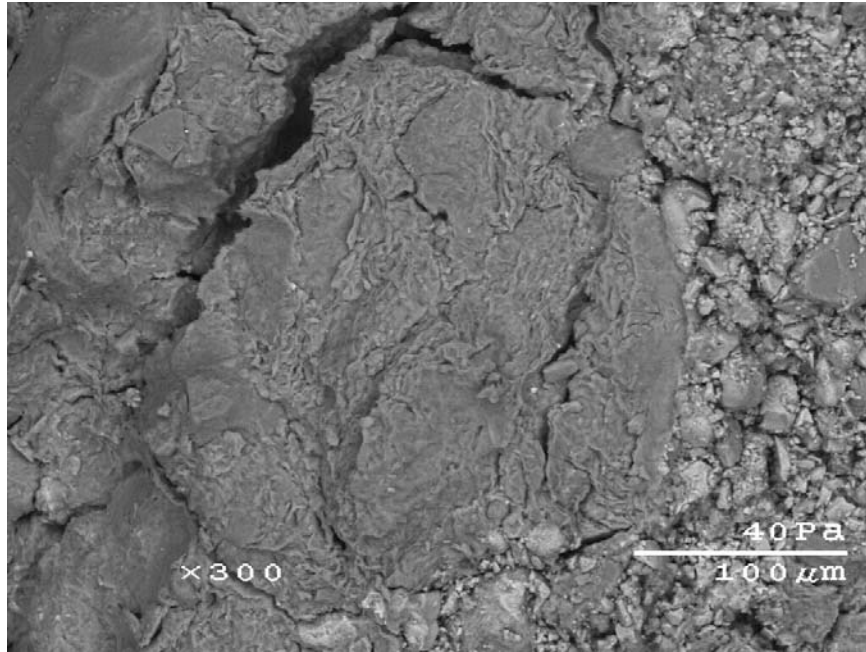
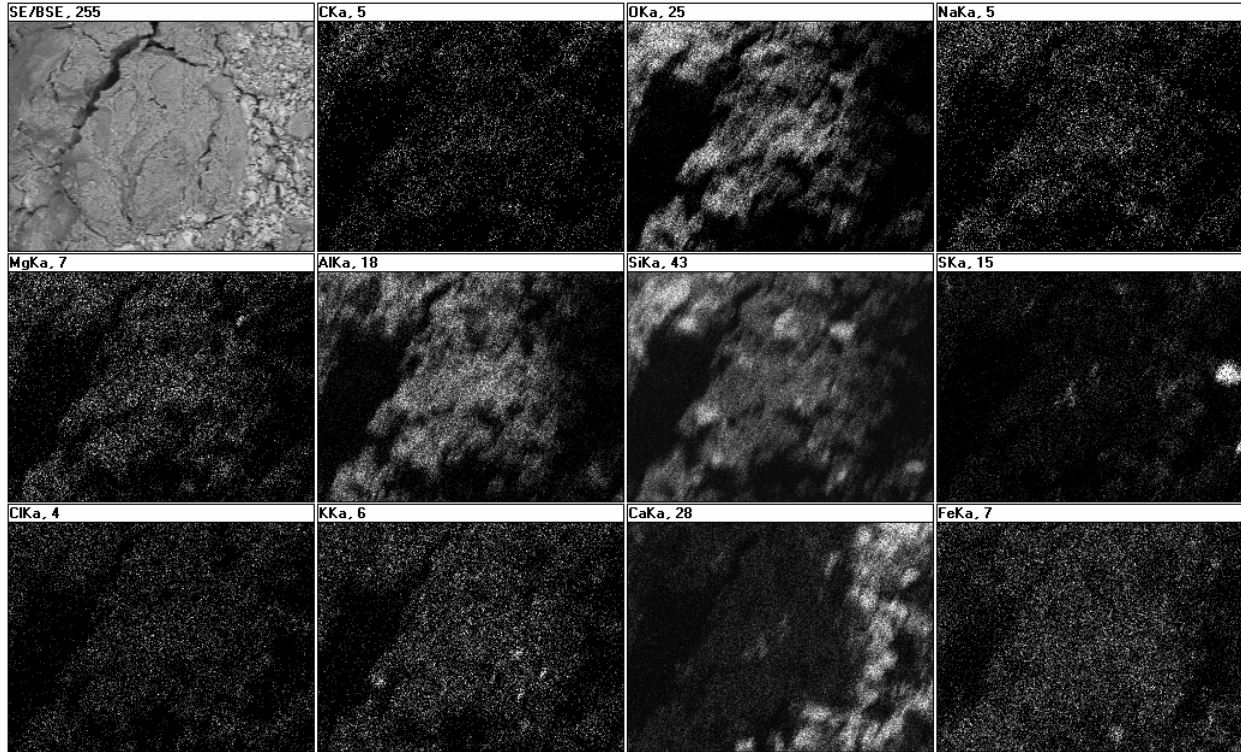


**Soil L with lime and type I II cement, magnified 3000x**





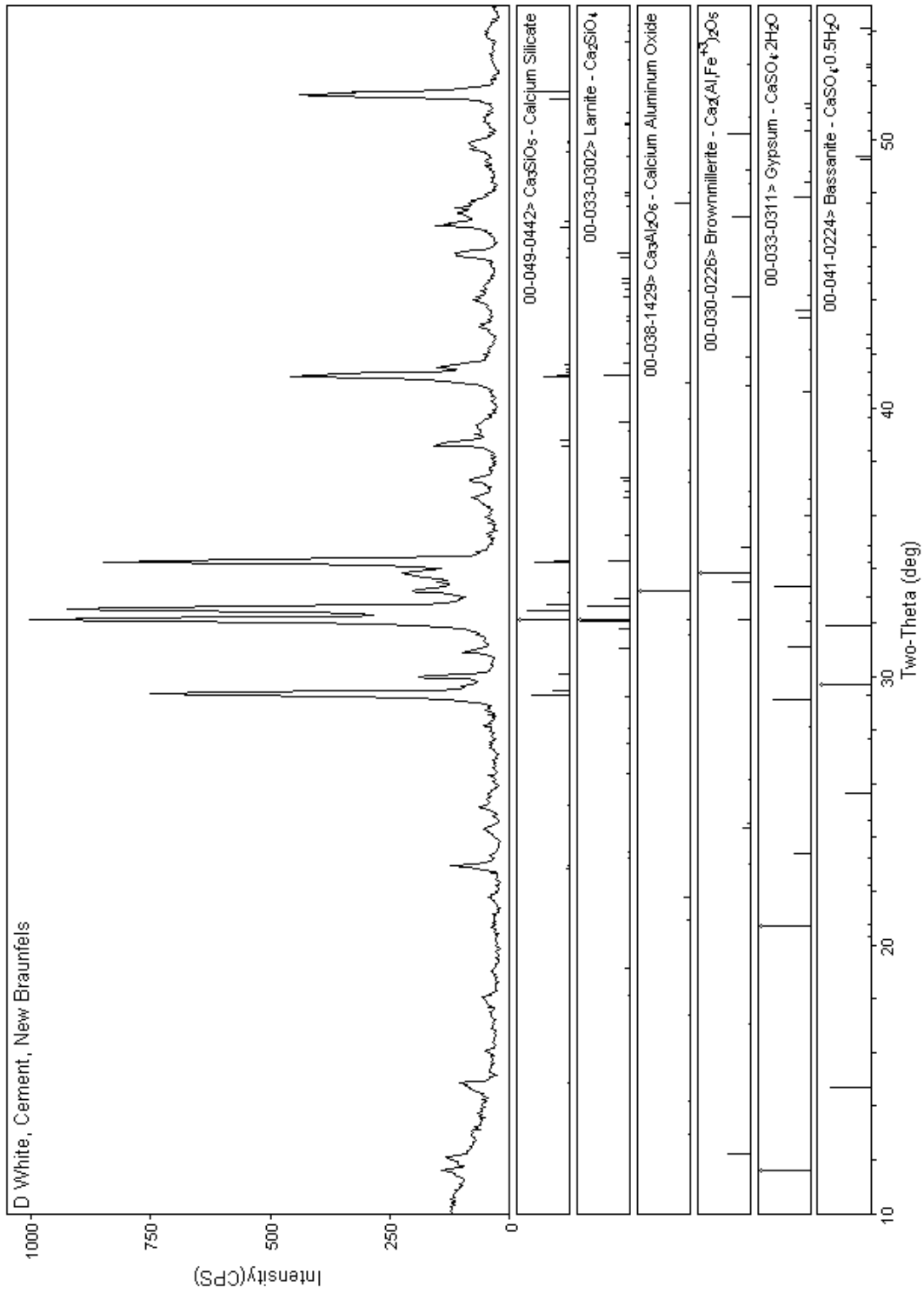
**Top: EDS map of high sulfate soil. Bottom: SEM image of sample mapped at 25x magnification**



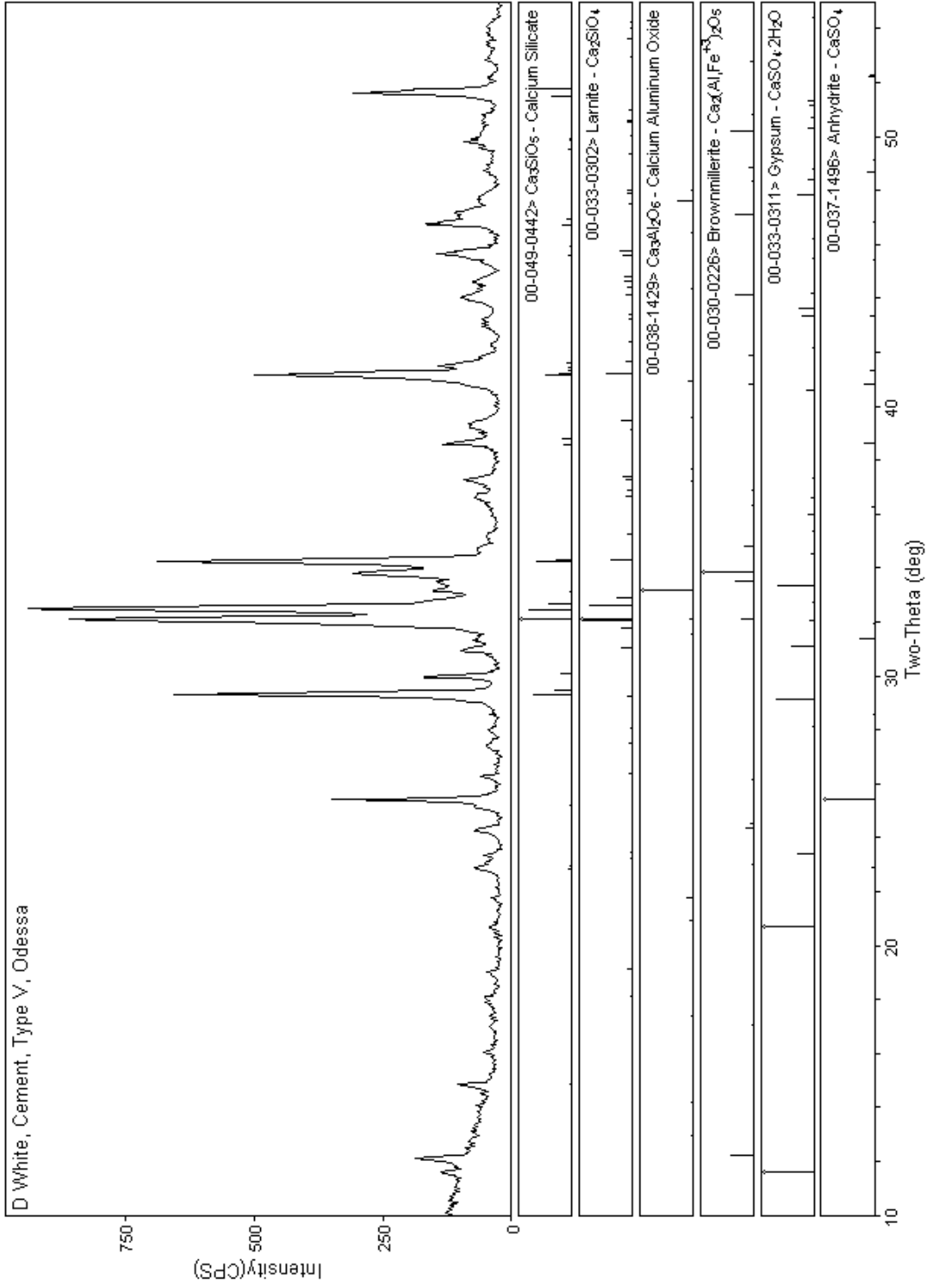
**Above: EDS map of S soil type V cement batch. Below: SEM image S soil and type V cement at 300x magnification.**



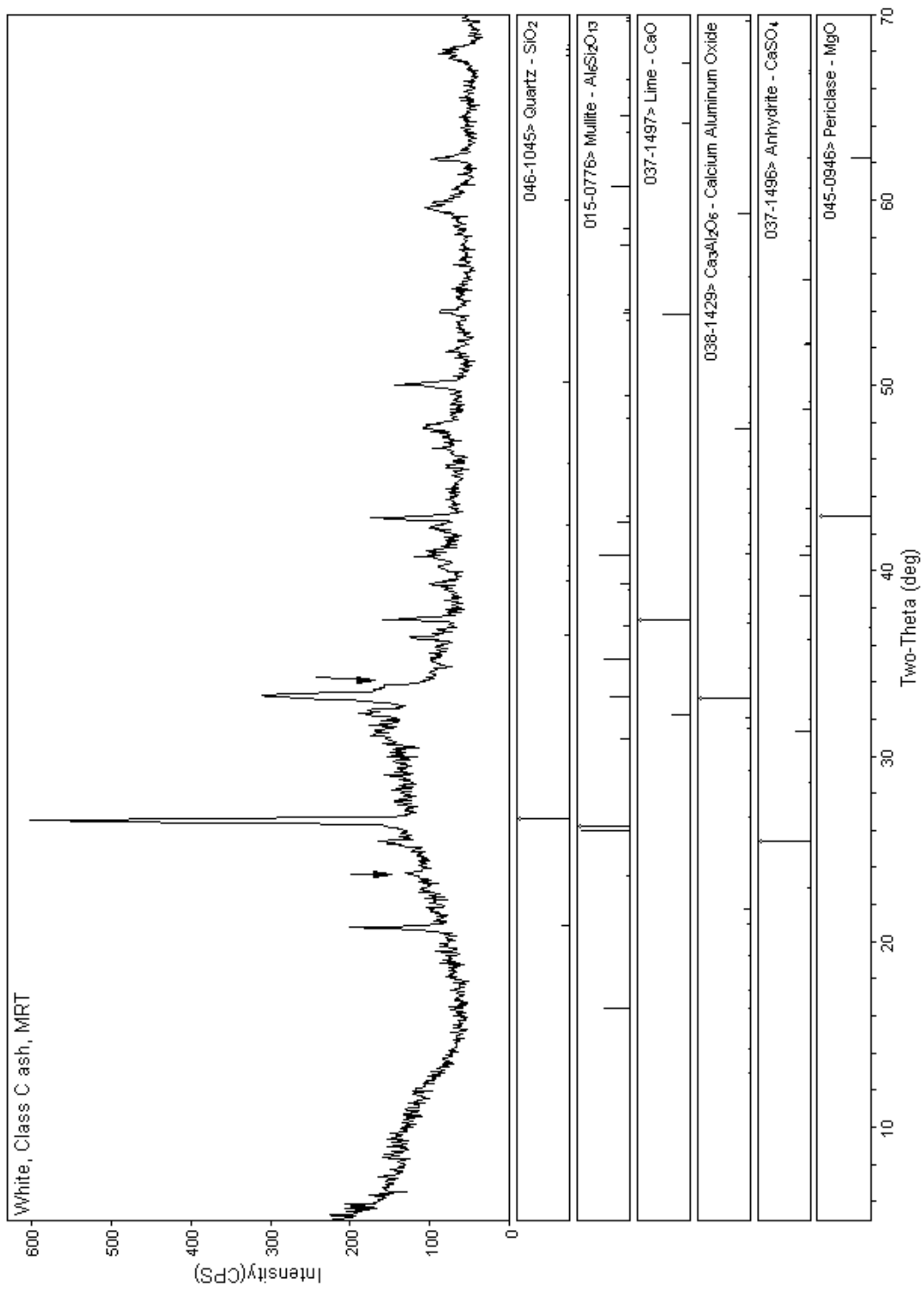
# APPENDIX E. STABILIZER XRD DIFFRACTOGRAMS



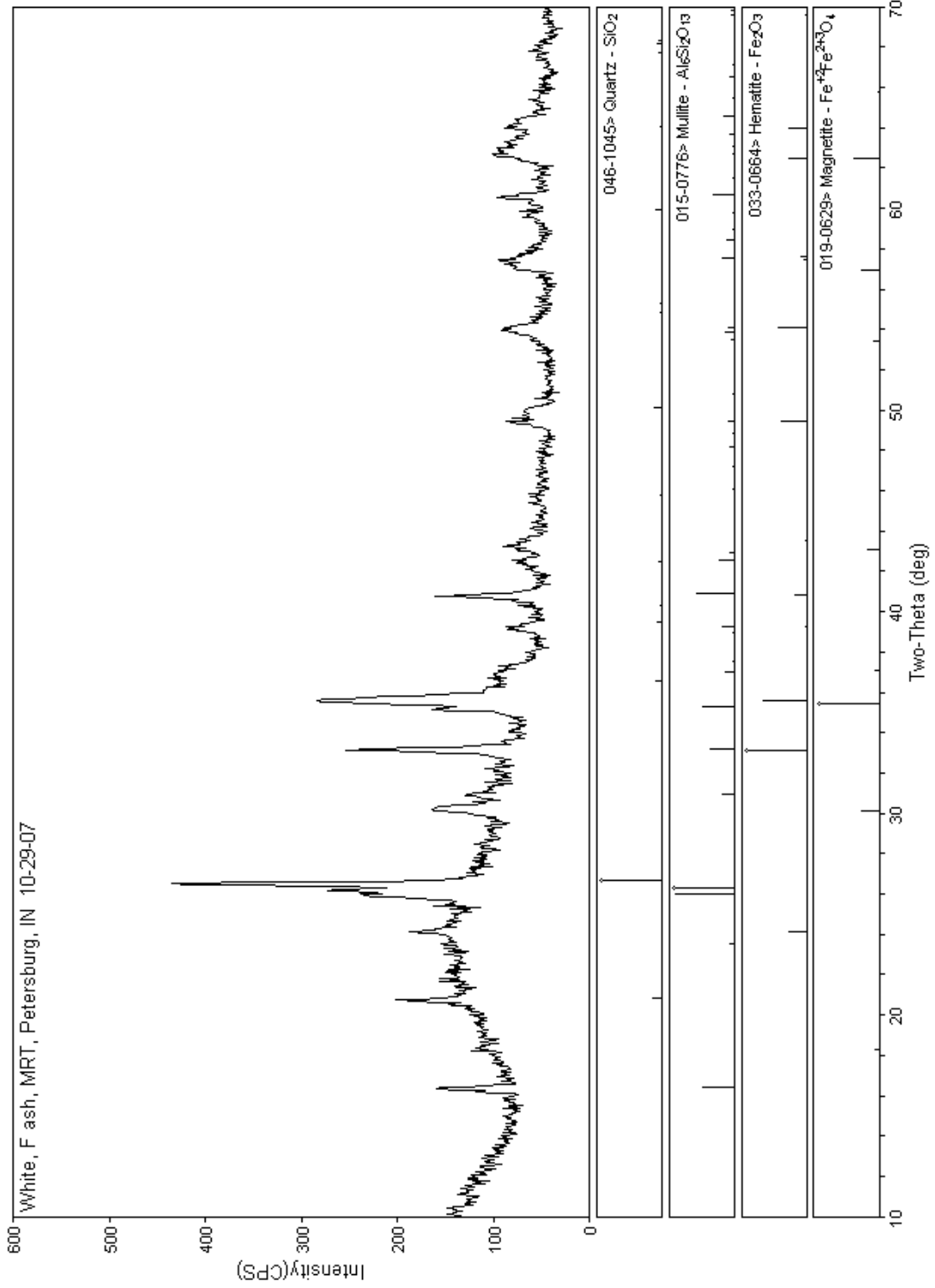
X-ray diffractogram of New Braunfels Type I II cement



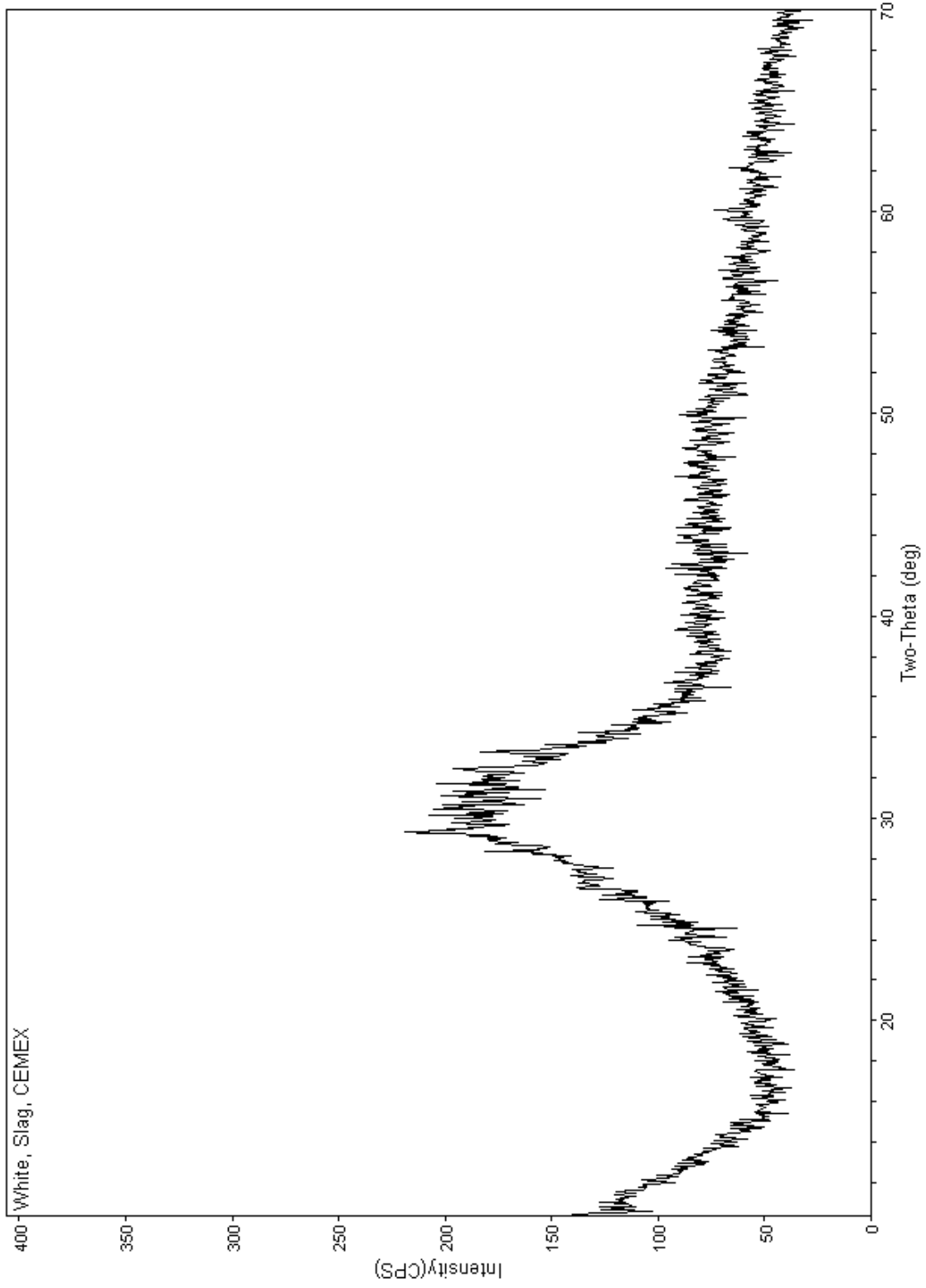
**X-ray diffractogram of Odessa type V cement**



**X-ray diffractogram of. Class C fly ash**

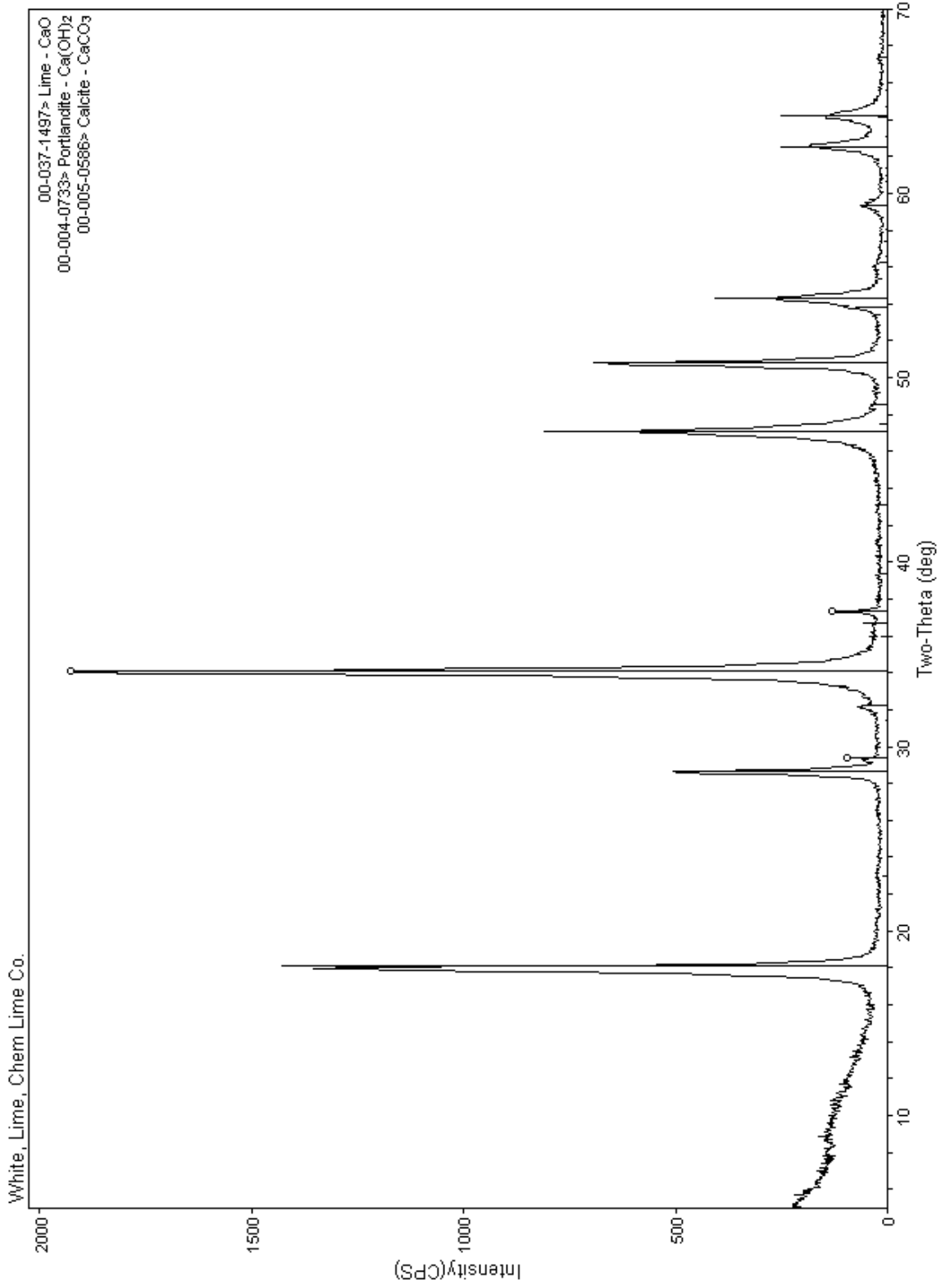


**X-ray diffractogram of. Class F fly ash**



**X-ray diffractogram of slag**

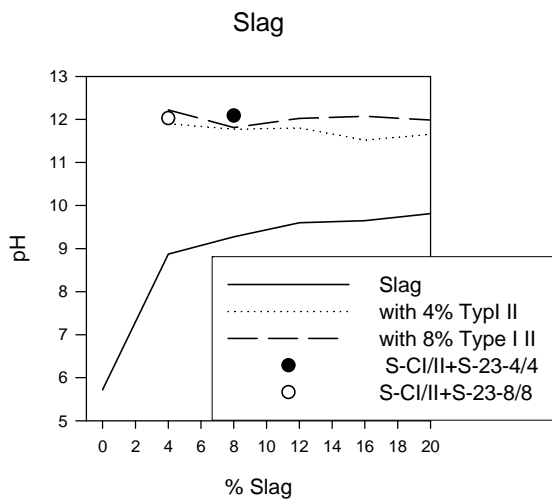
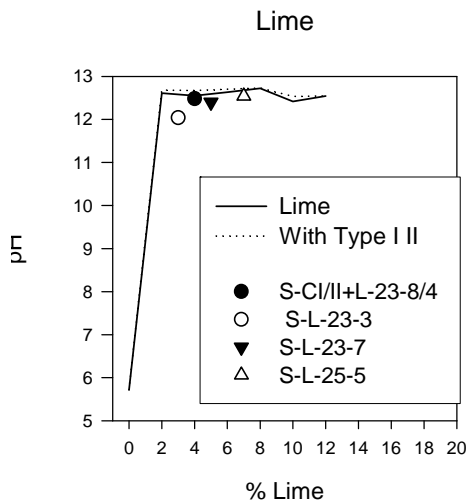
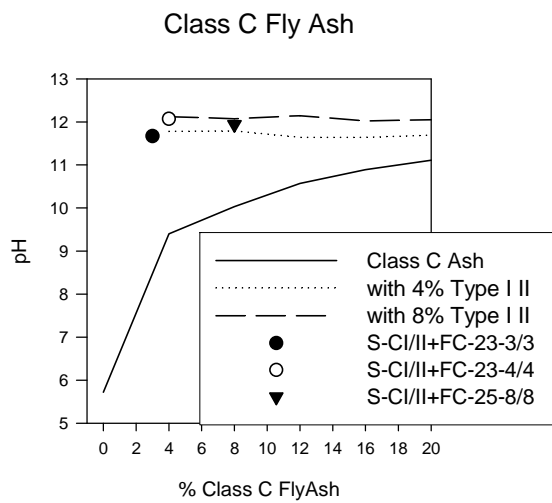
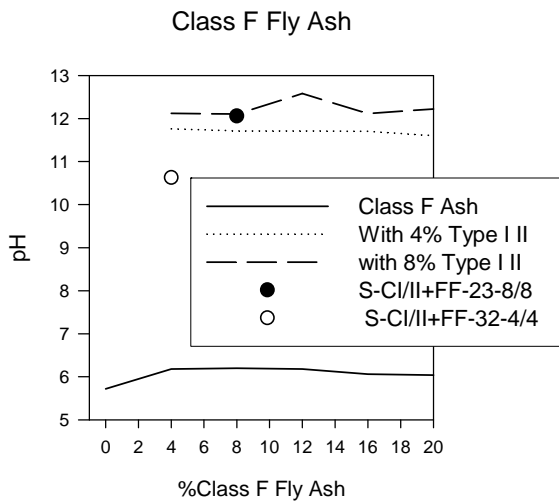
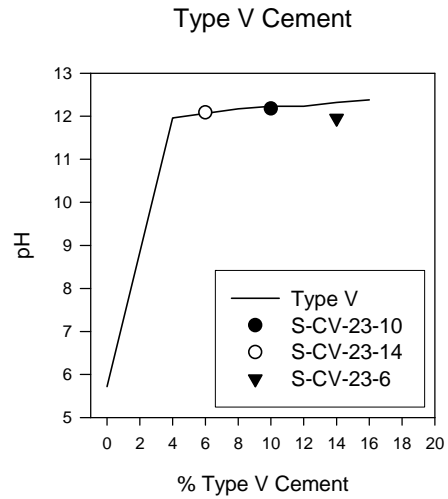
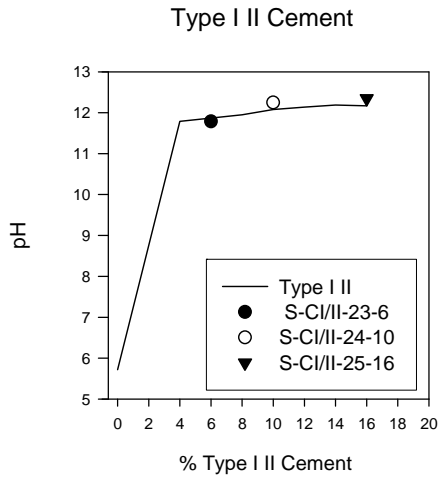




**X-ray diffractogram of. slag**

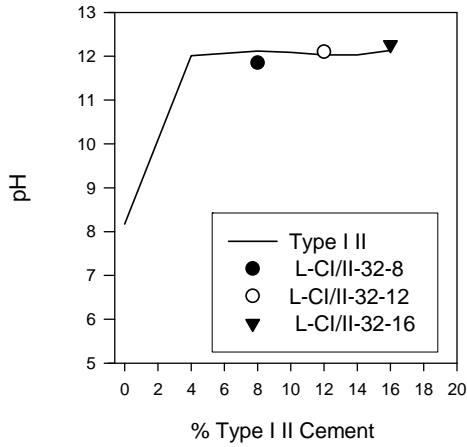
# APPENDIX F. pH MEASUREMENTS

## S Soil pH Values

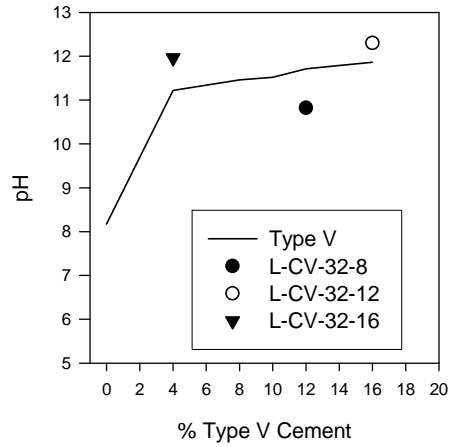


# L Soil ph Values

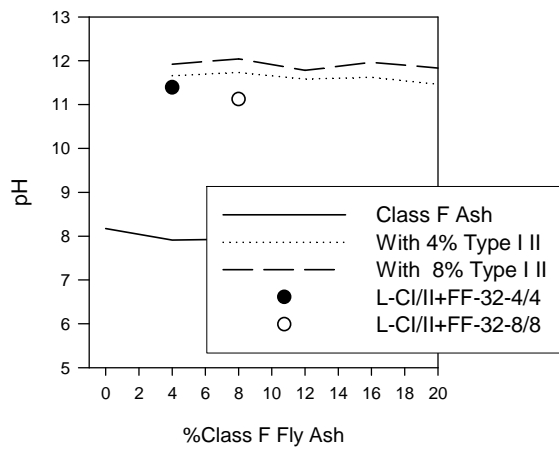
## Type I II Cement



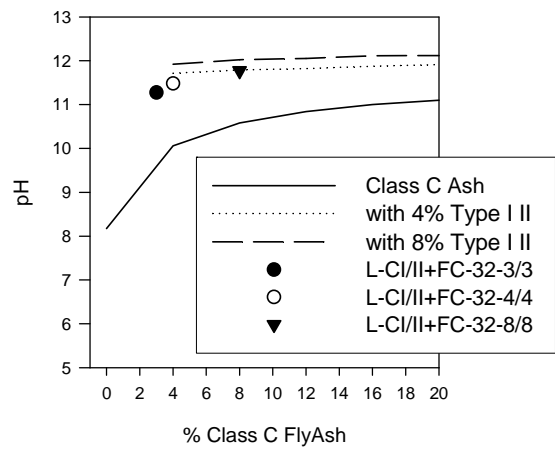
## Type V Cement



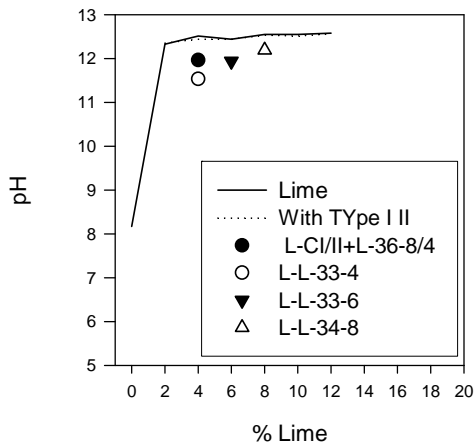
## Class F Fly Ash



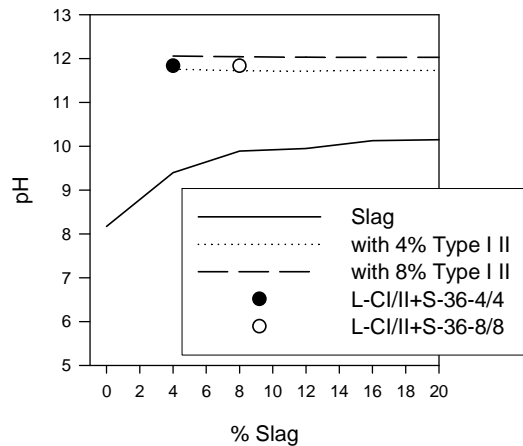
## Class C Fly Ash



## Lime

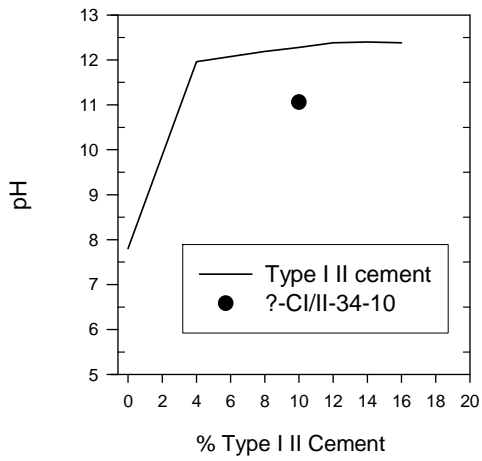


## Slag

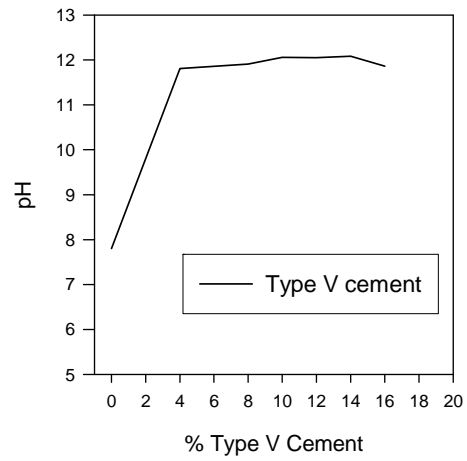


# ? Soil pH Values

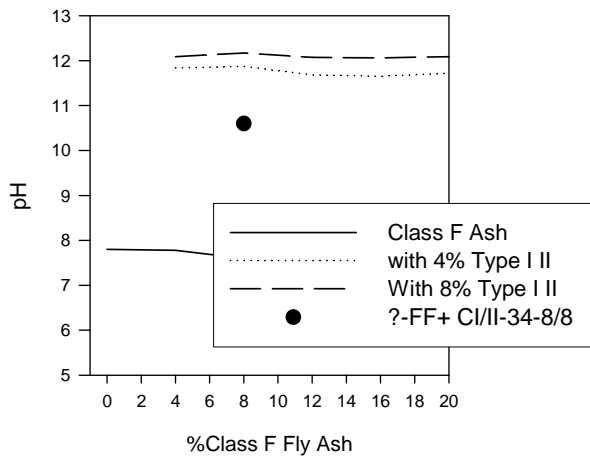
Type I II Cement



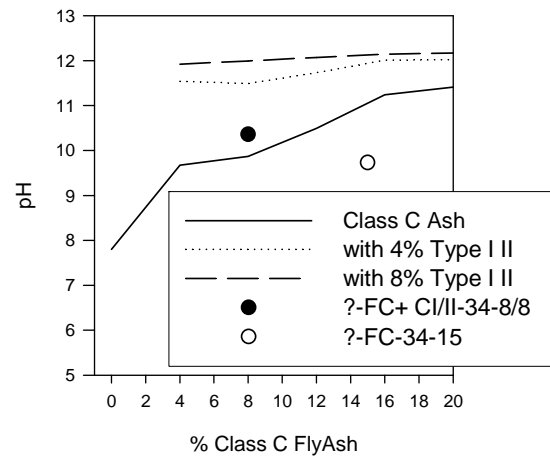
Type V Cement



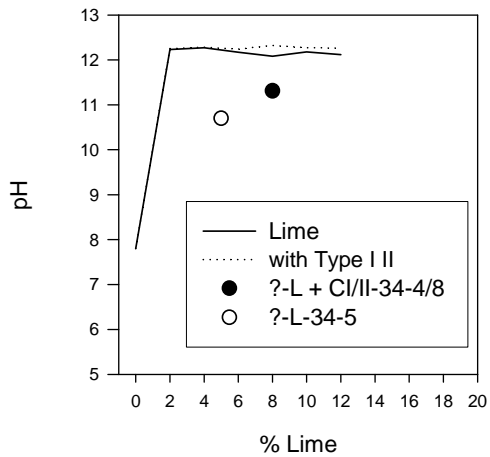
Class F Fly Ash



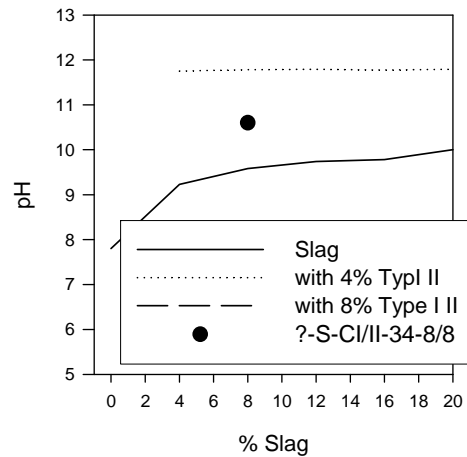
Class C Fly Ash



Lime



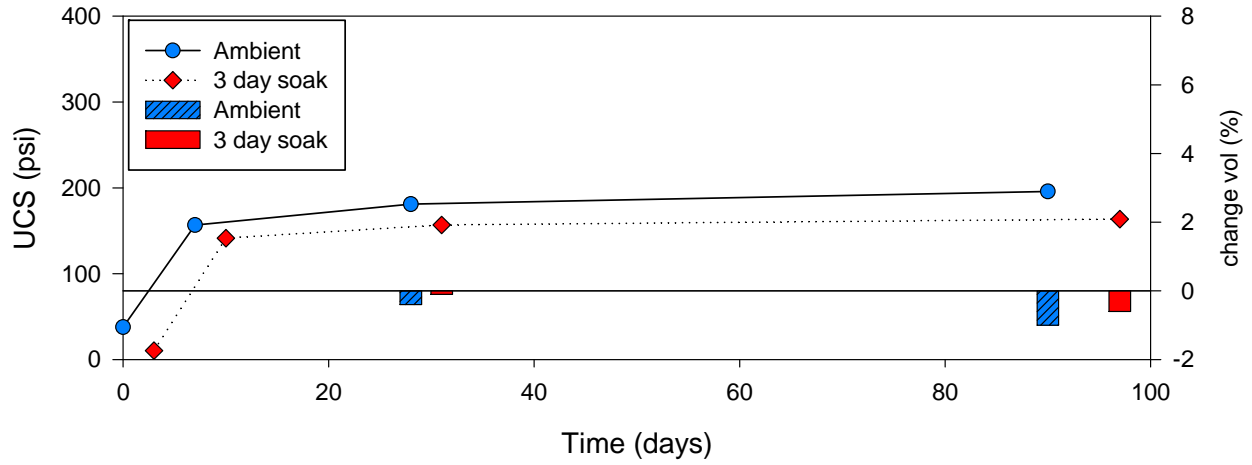
Slag



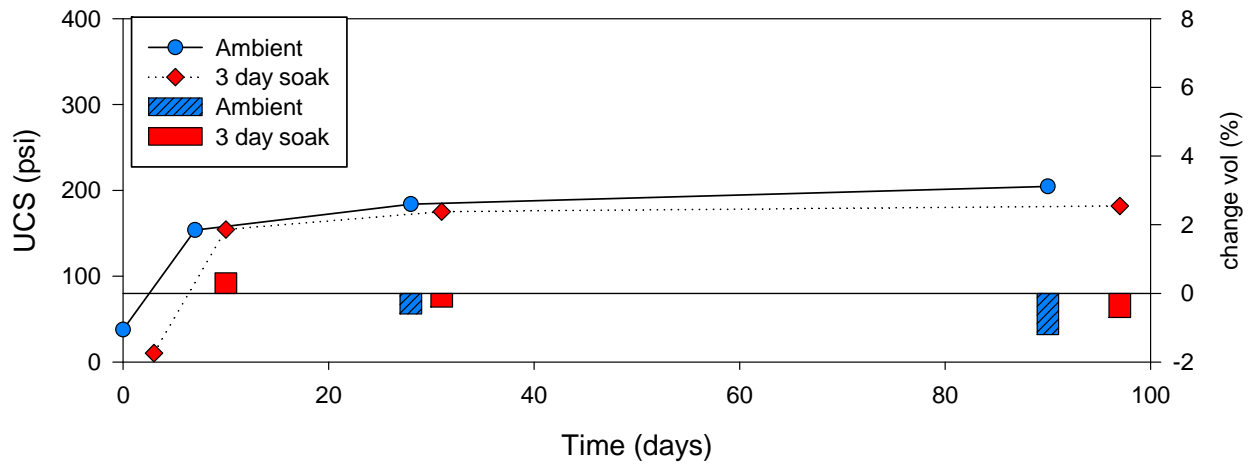


# APPENDIX G. STRENGTH CURVES WITH VOLUMETRIC SWELL

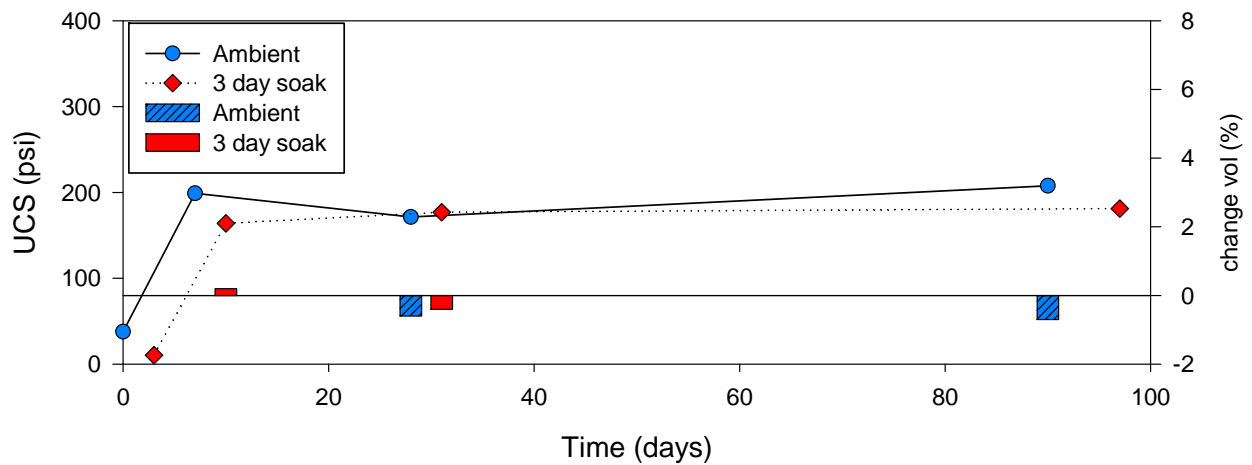
## L Soil Type I II Cement 8%



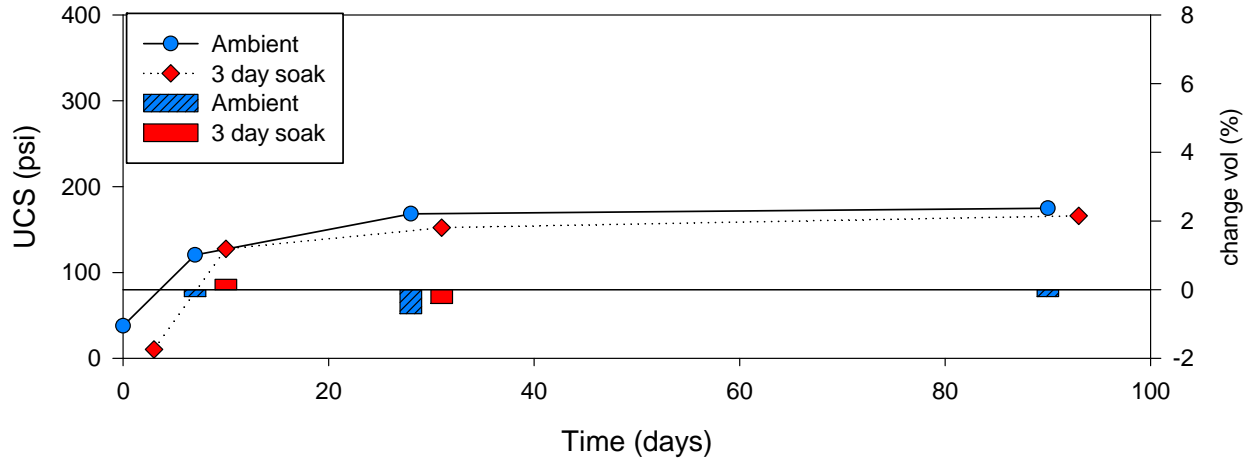
## L Soil Type I II Cement 12%



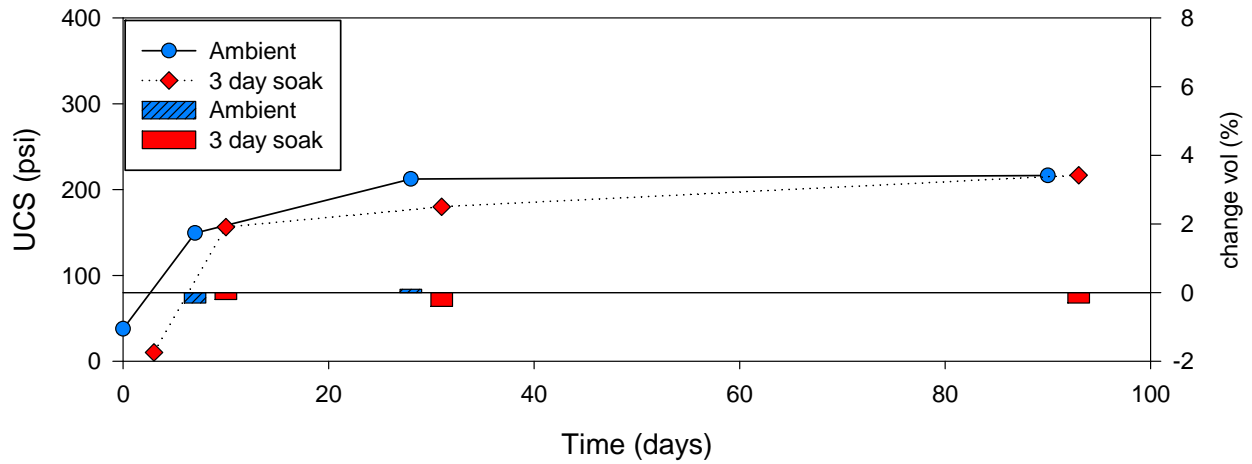
## L Soil Type I II Cement 16%



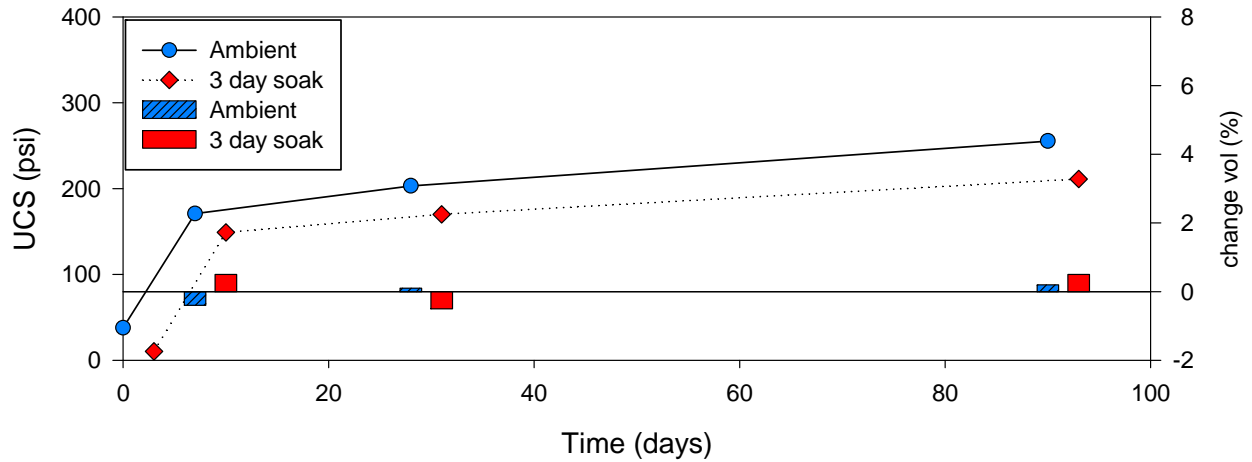
### L Soil Type V Cement 8%



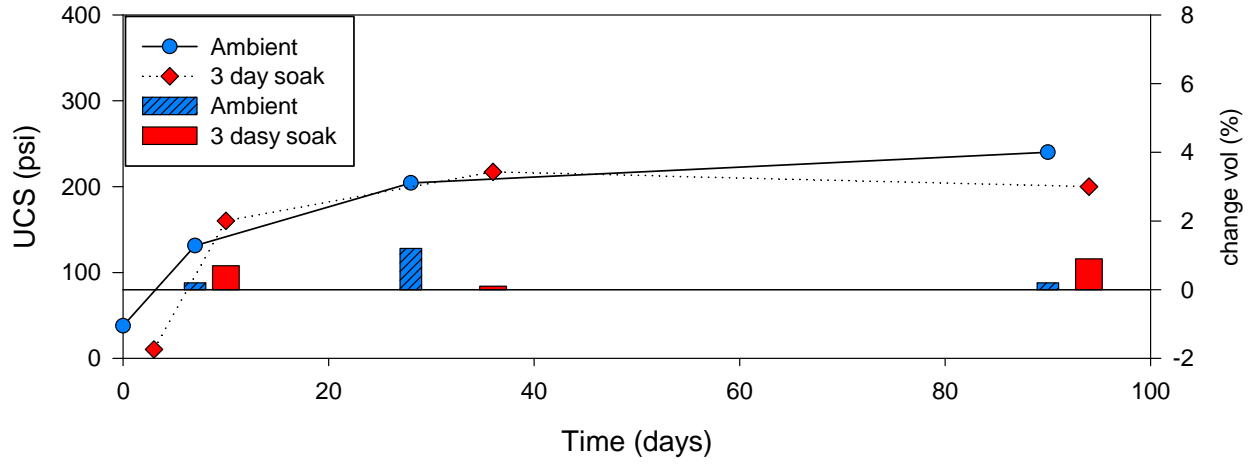
### L Soil Type V Cement 12%



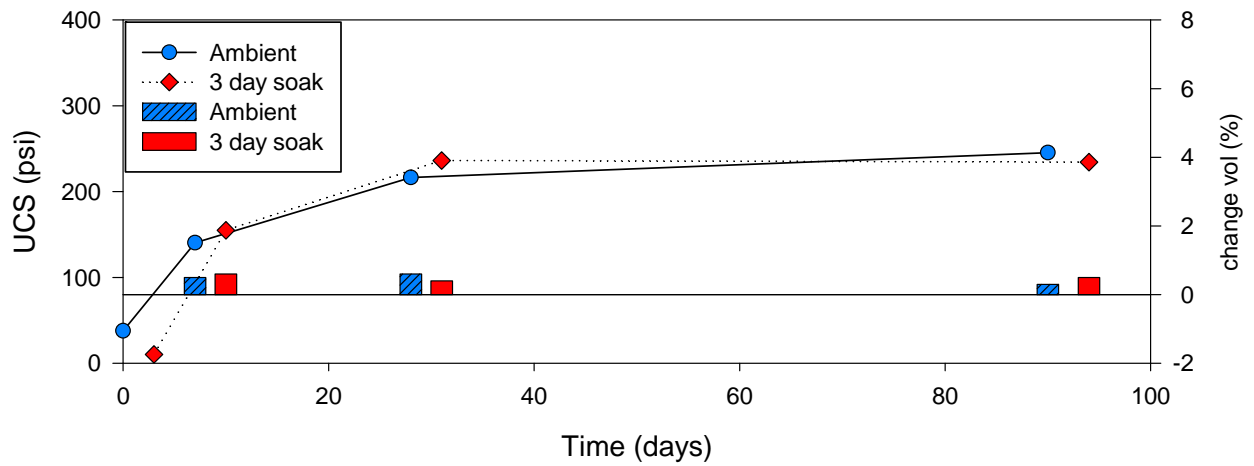
### L Soil Type V Cement 16%



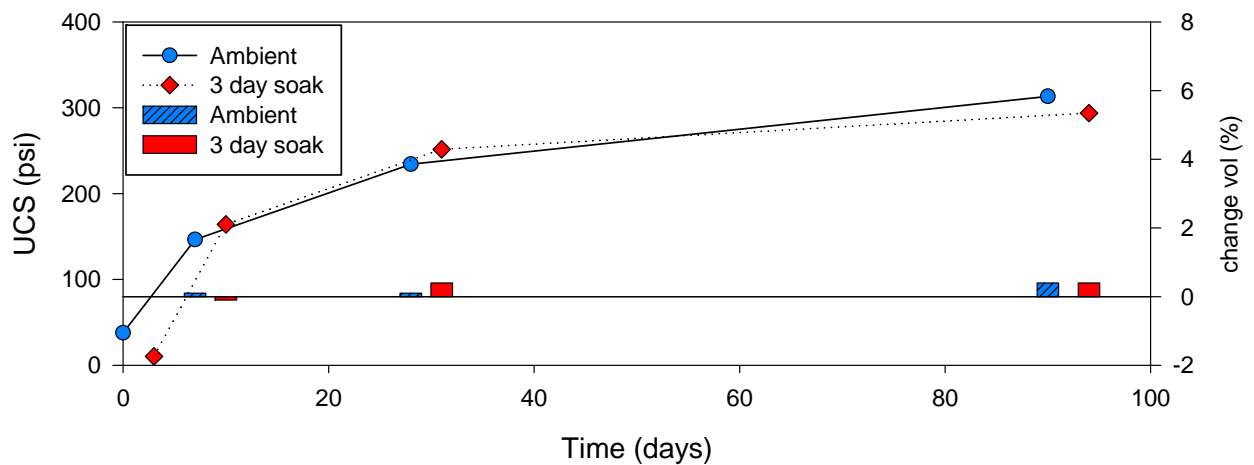
### L Soil Lime 4%



### L Soil Lime 6%

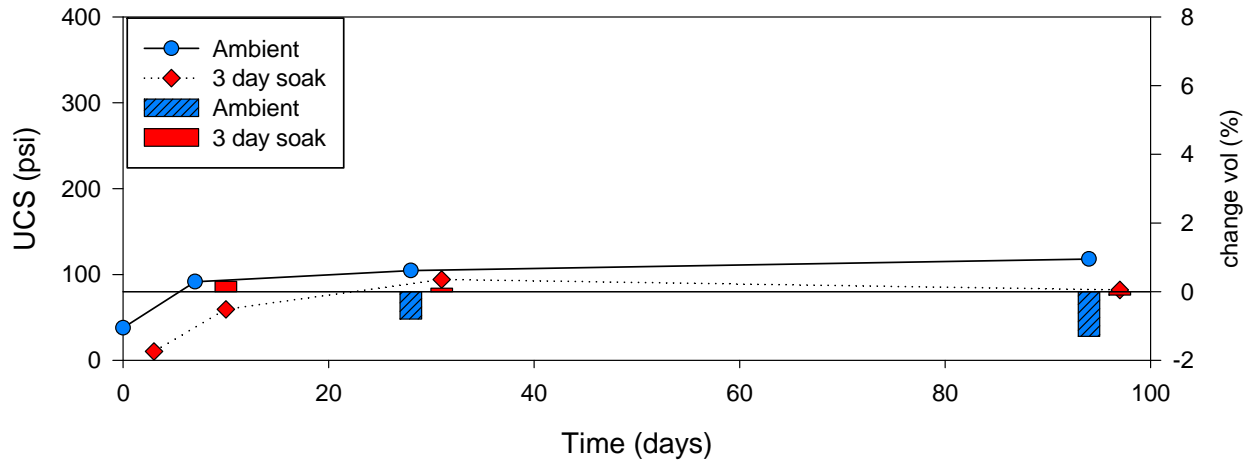


### L Soil Lime 8%

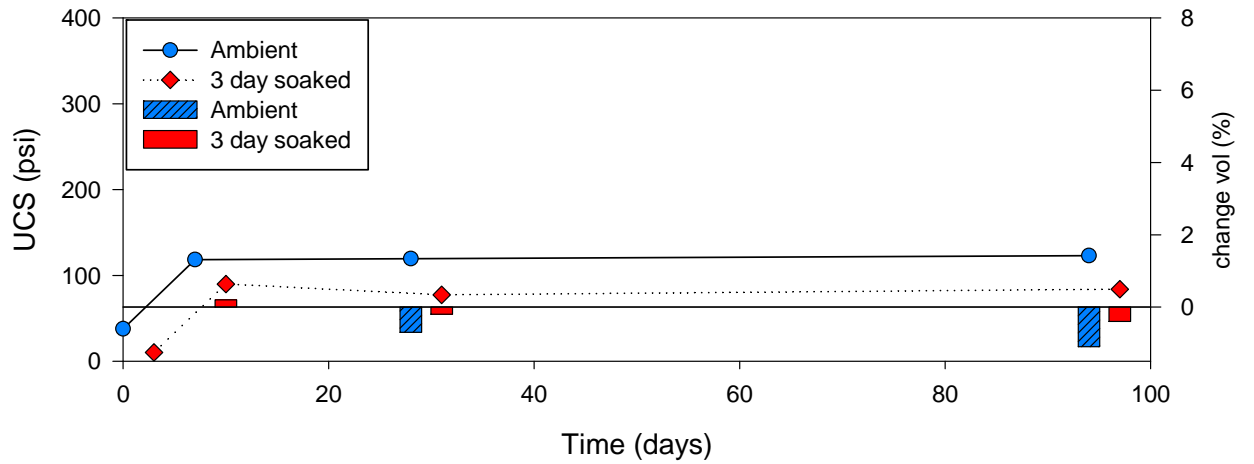




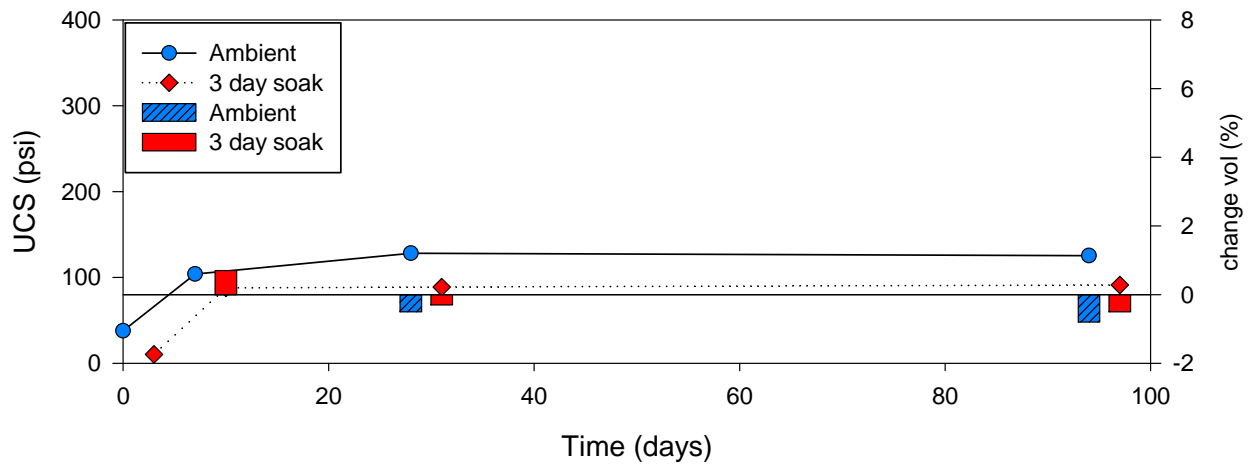
### L Soil Class C Fly Ash 10%



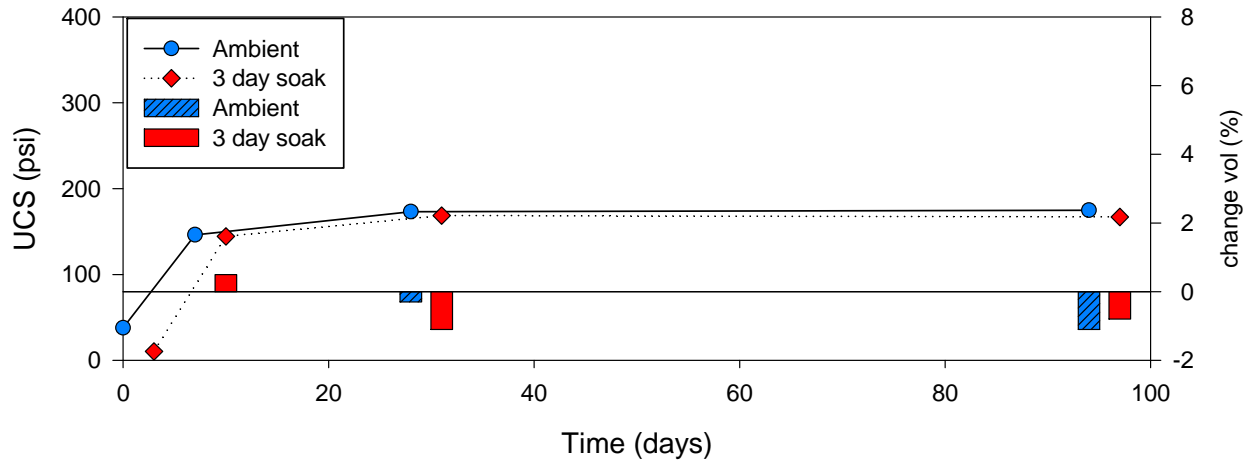
### L Soil Class C Fly Ash 15%



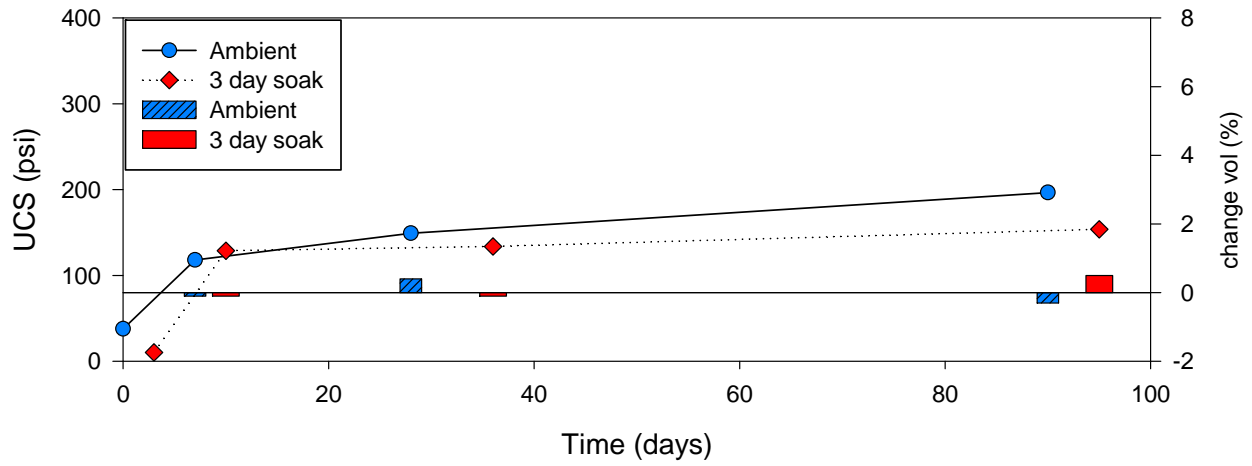
### L Soil Class C Fly Ash 20%



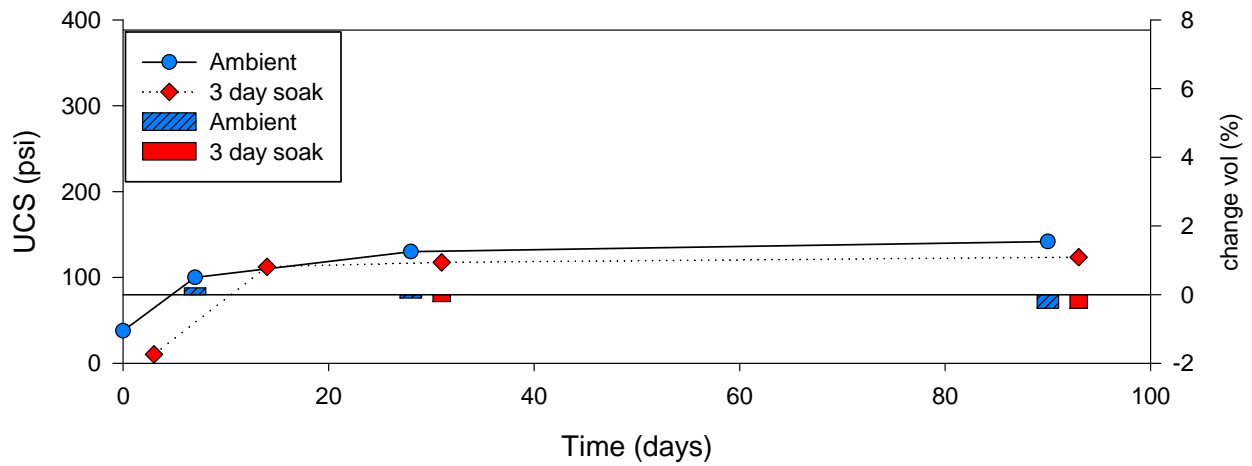
### L Soil Cement 8% with Fly Ash C 8%



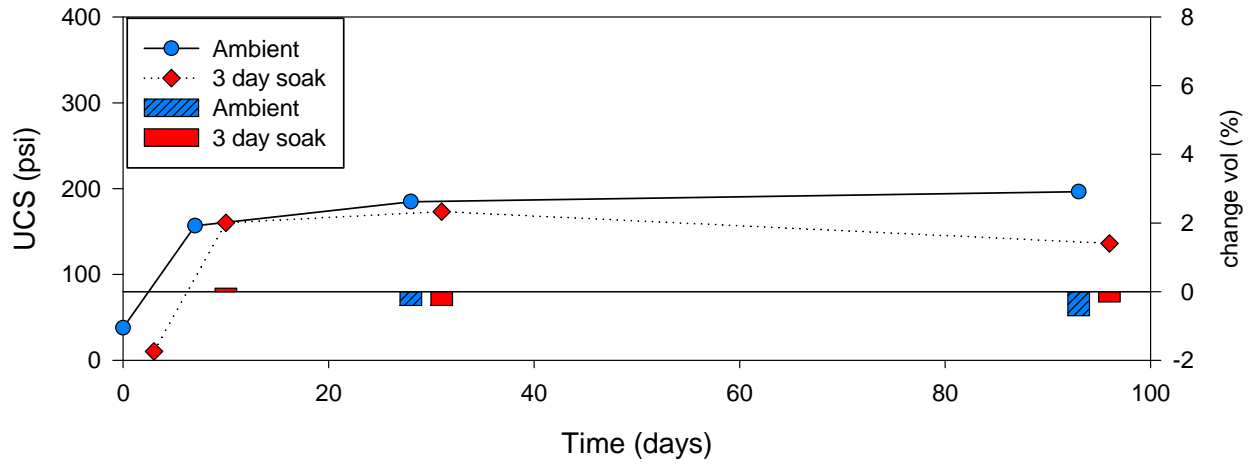
### L Soil Cement 4% with Fly Ash C 4%



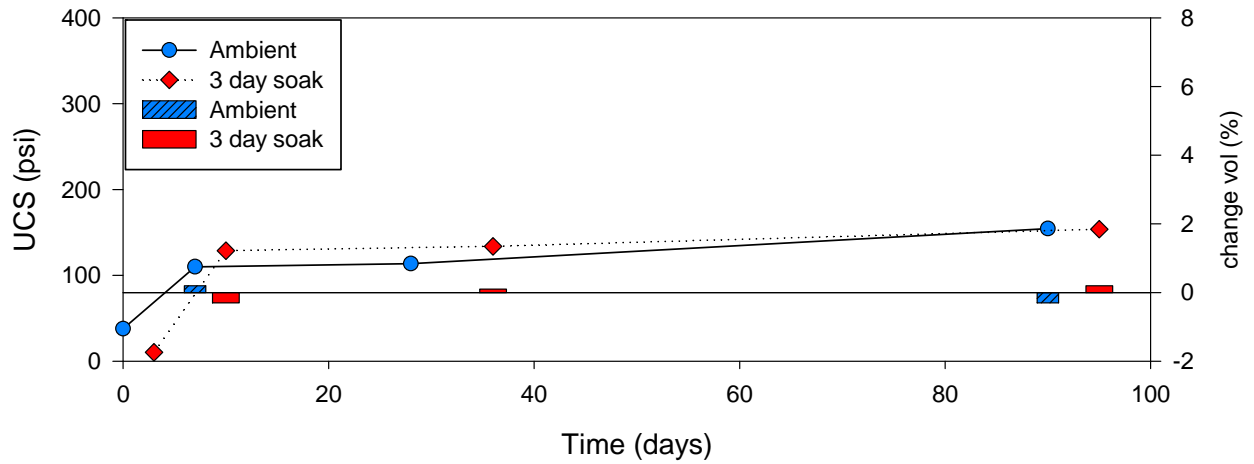
### L Soil Cement 3% with Fly Ash C 3%



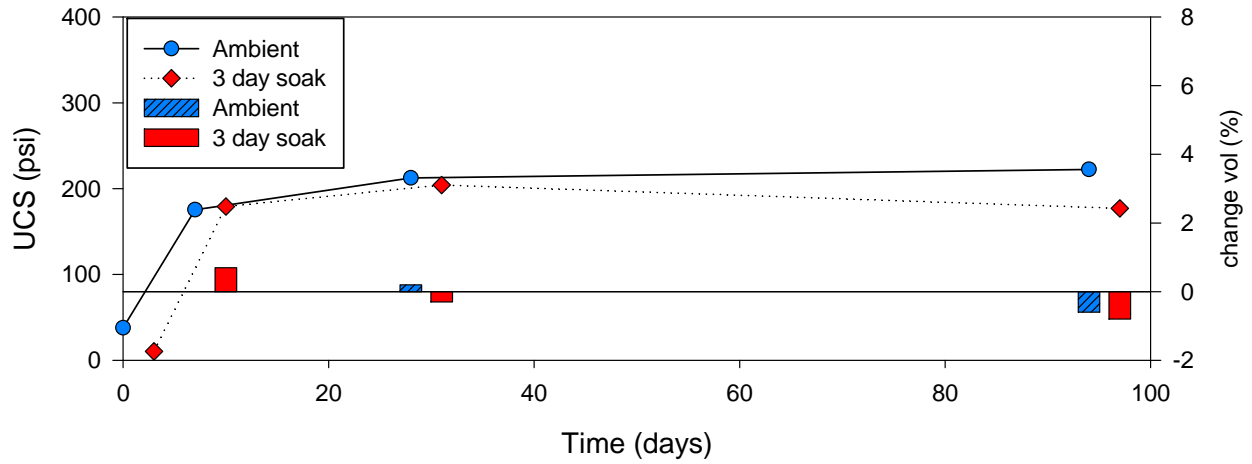
### L Soil Cement 8% with Fly Ash F 8%



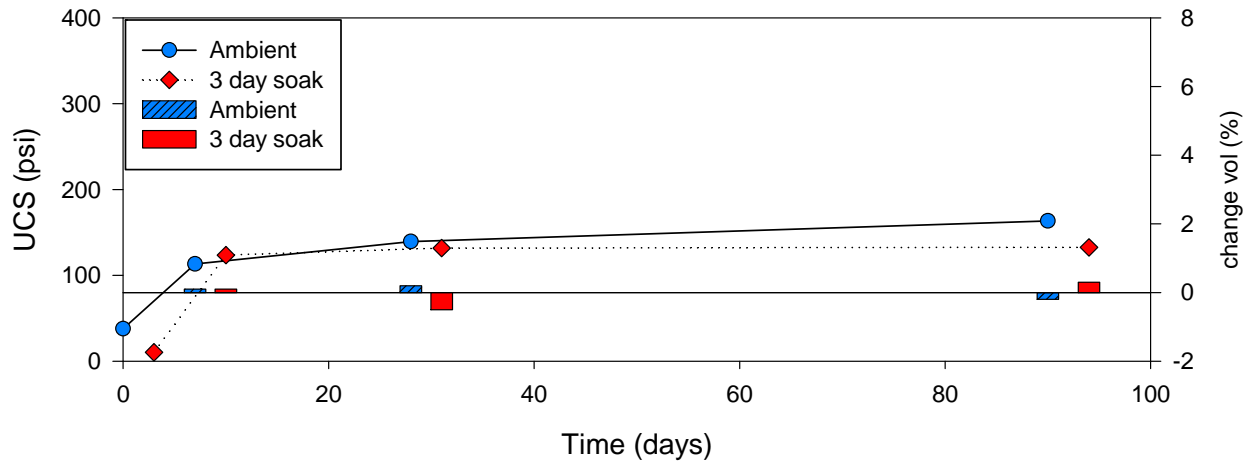
### L Soil Cement 4% with Fly Ash F 4%



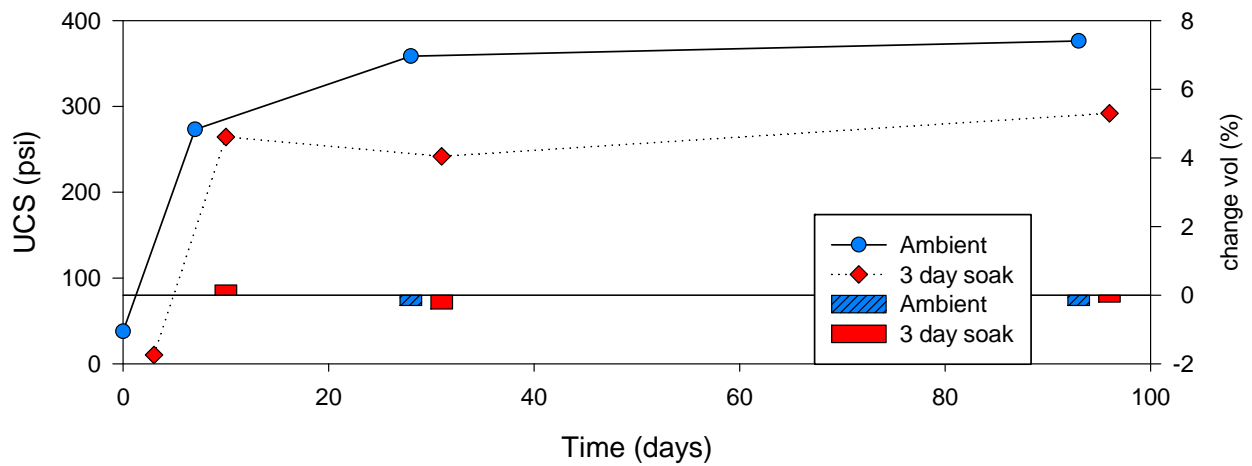
### L Soil Cement 8% with Slag 8%



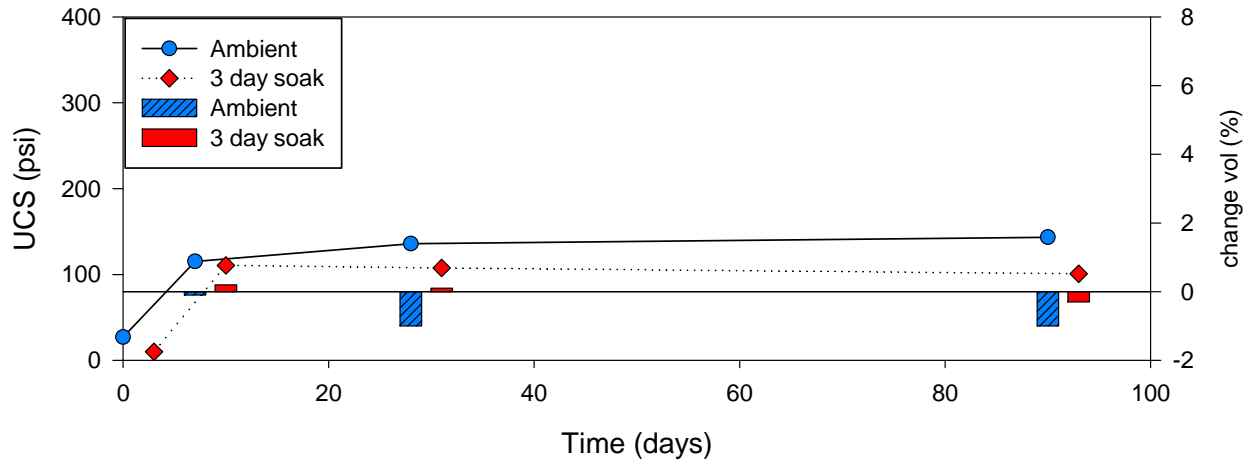
### L Soil Cement 4% with Slag 4%



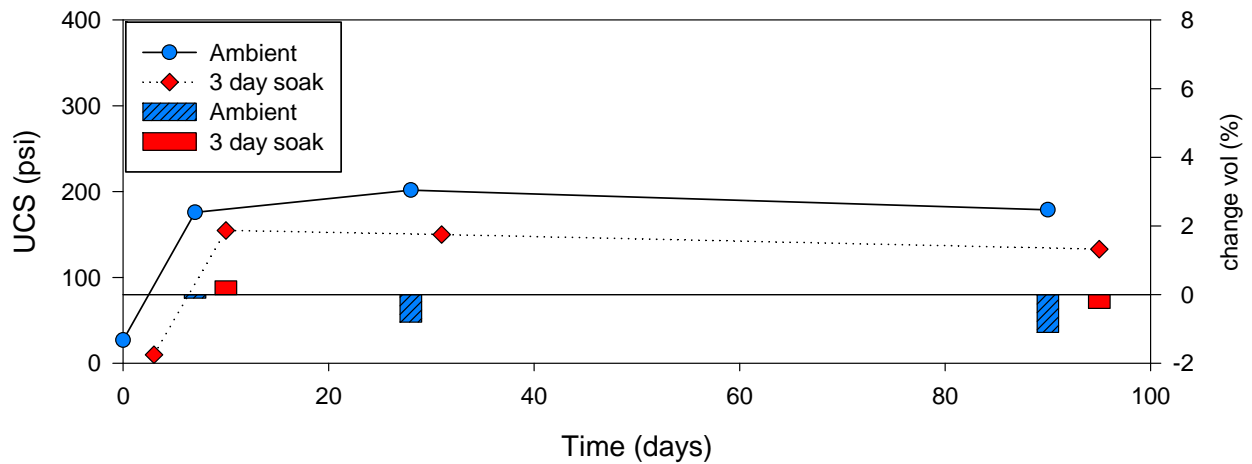
### L Soil Cement 8% with Lime 4%



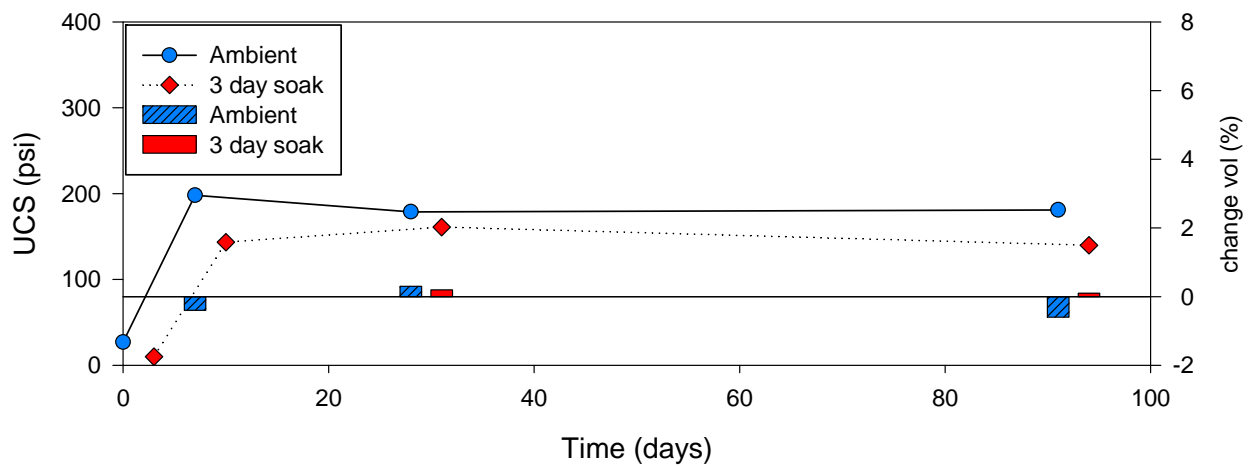
### S Soil Type I II Cement 6%



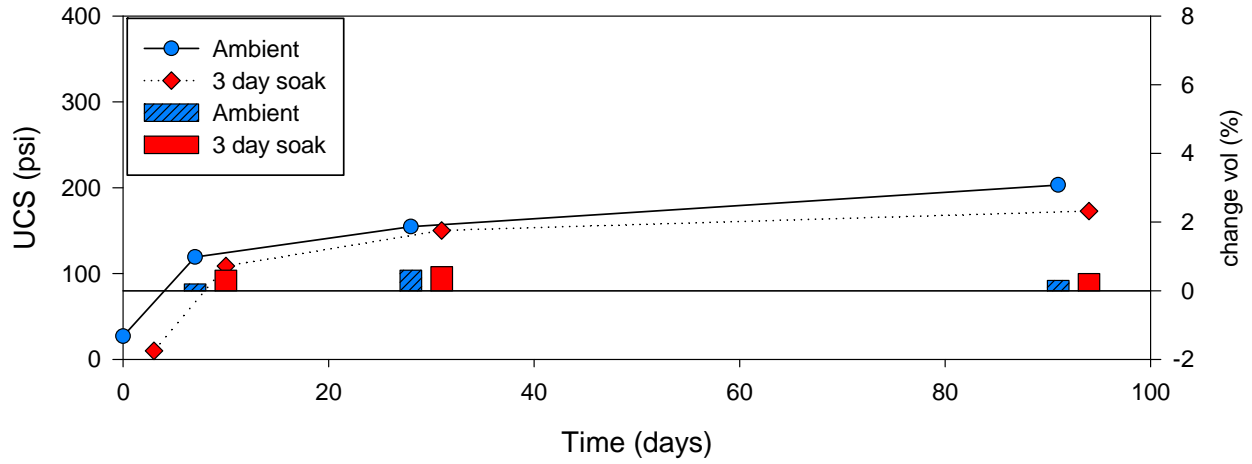
### S Soil Type I II Cement 10%



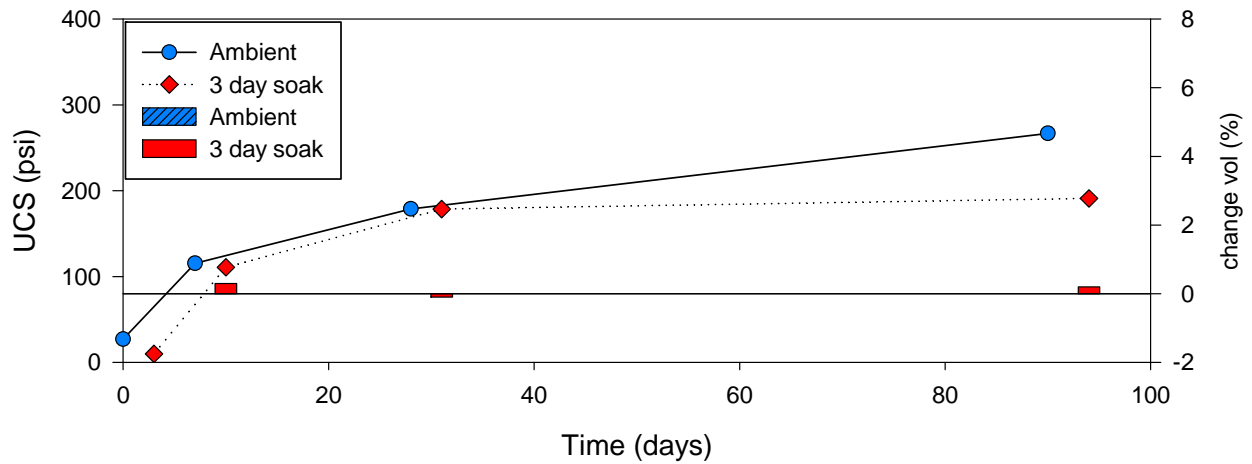
### S Soil Type I II Cement 16%



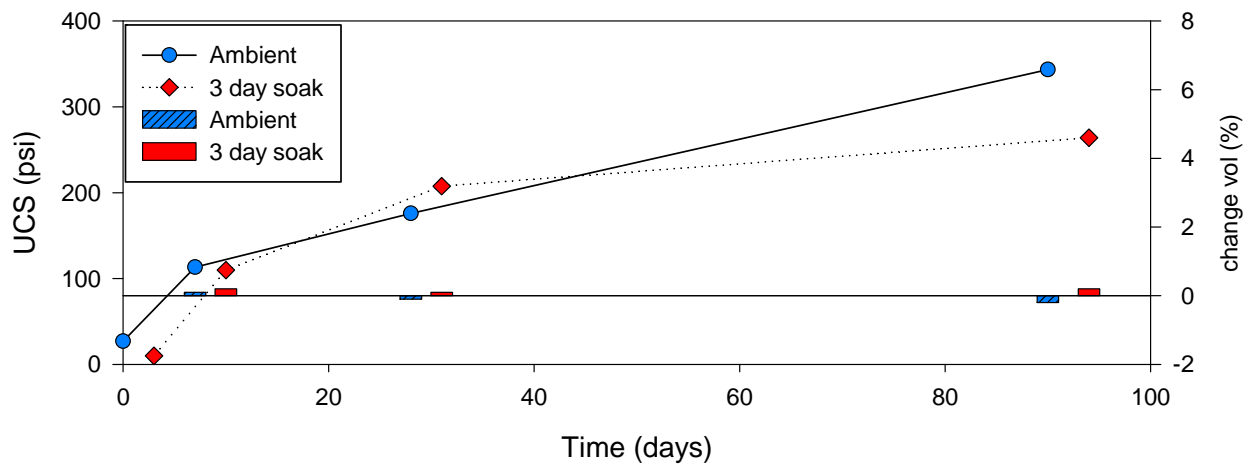
### S Soil Lime 3%



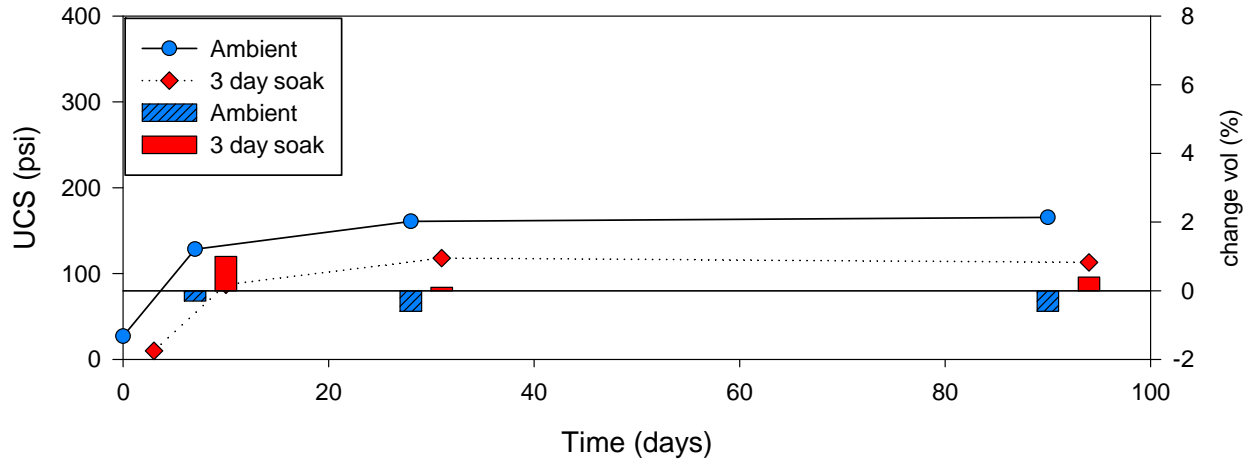
### S Soil Lime 5%



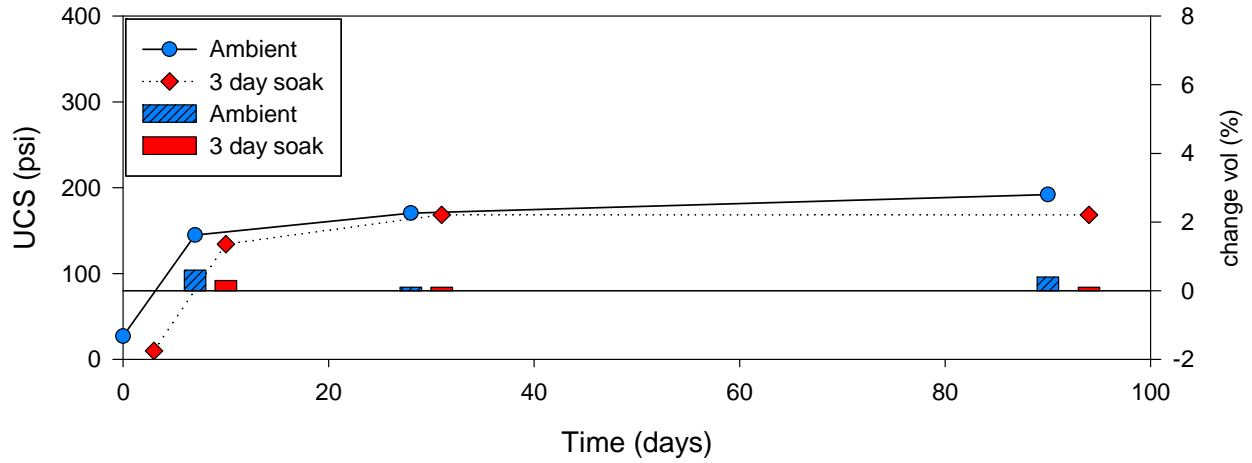
### S Soil Lime 7%



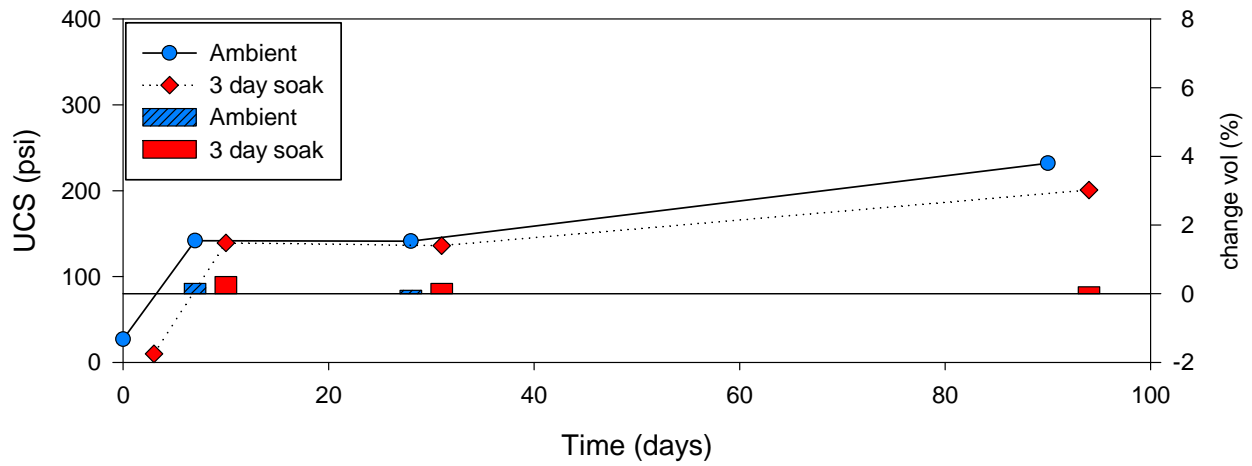
### S Soil Type V Cement 6%



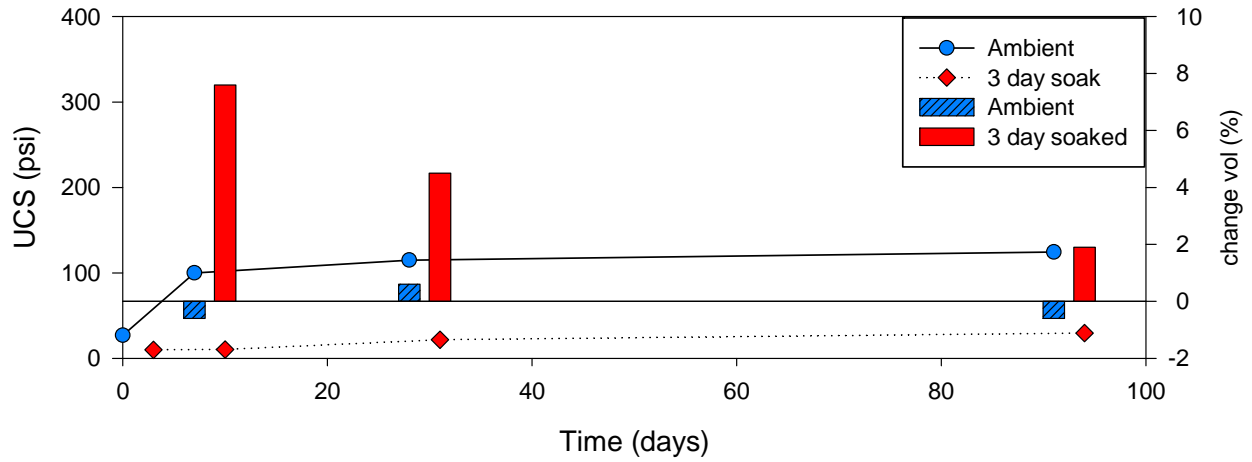
### S Soil Type V Cement 10%



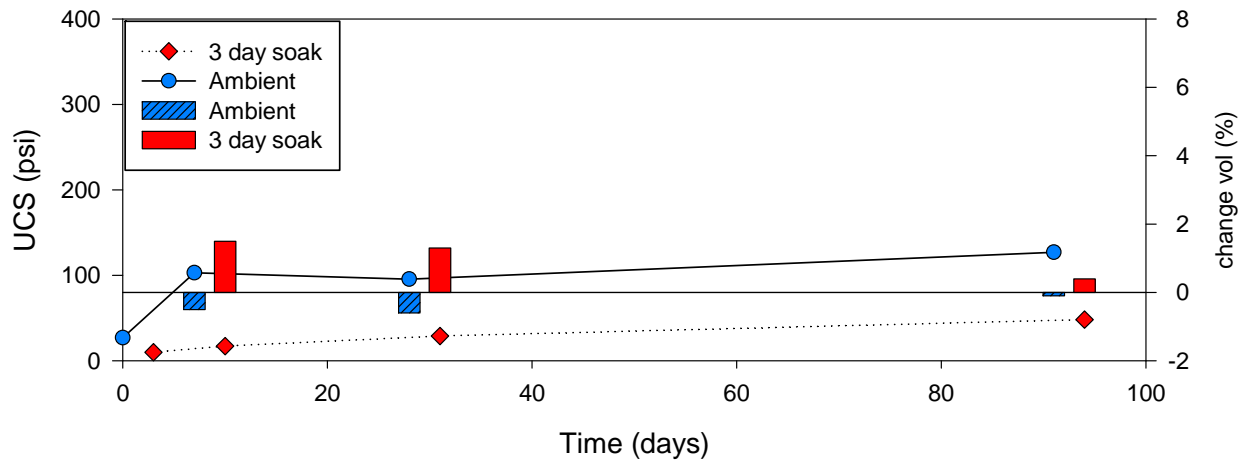
### S Soil Type V Cement 14%



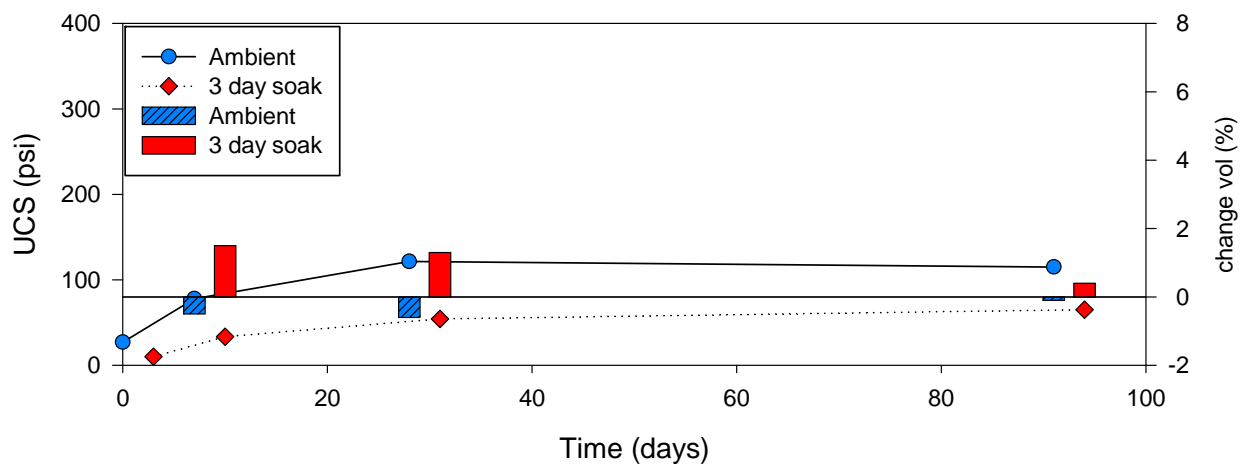
### S Soil Class C Fly Ash 10%



### S Soil Class C Fly Ash 15%

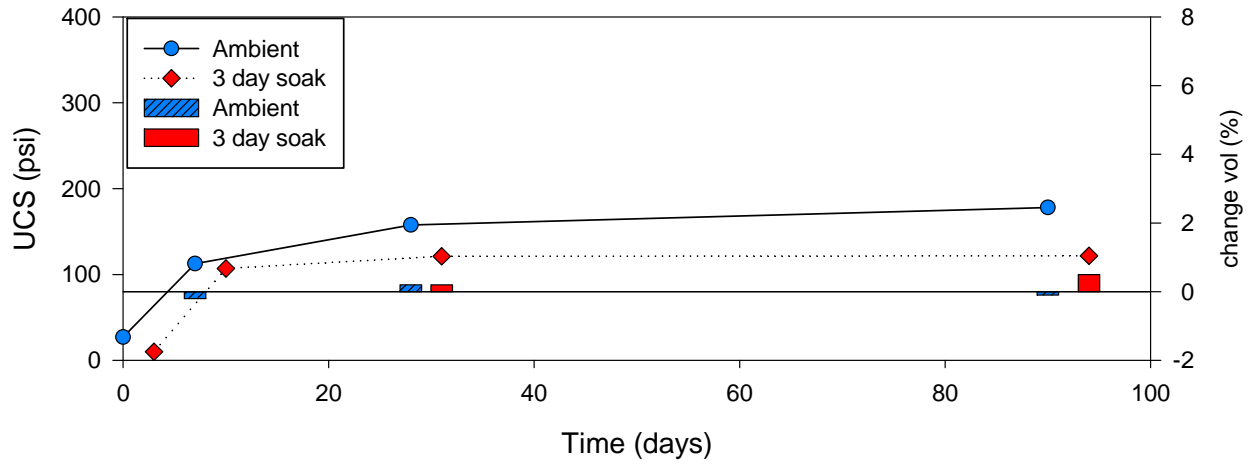


### S Soil Class C Fly Ash 20%

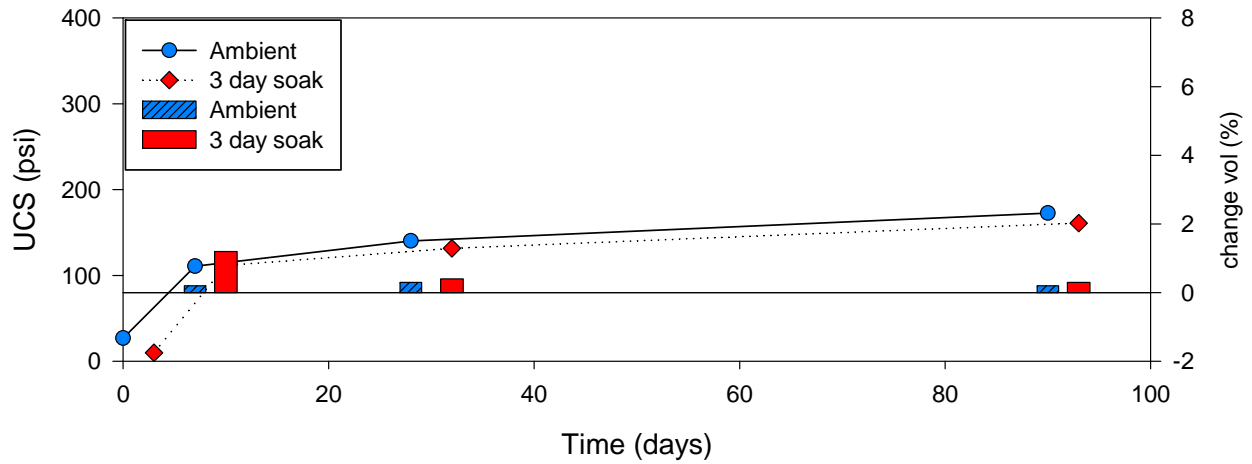




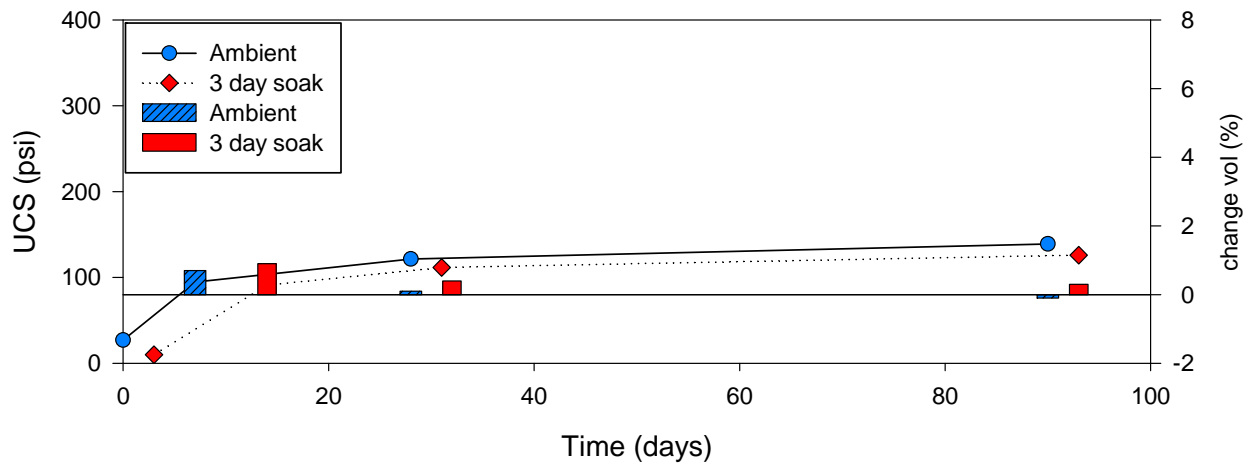
### S Soil Cement 8% with Fly Ash C 8%



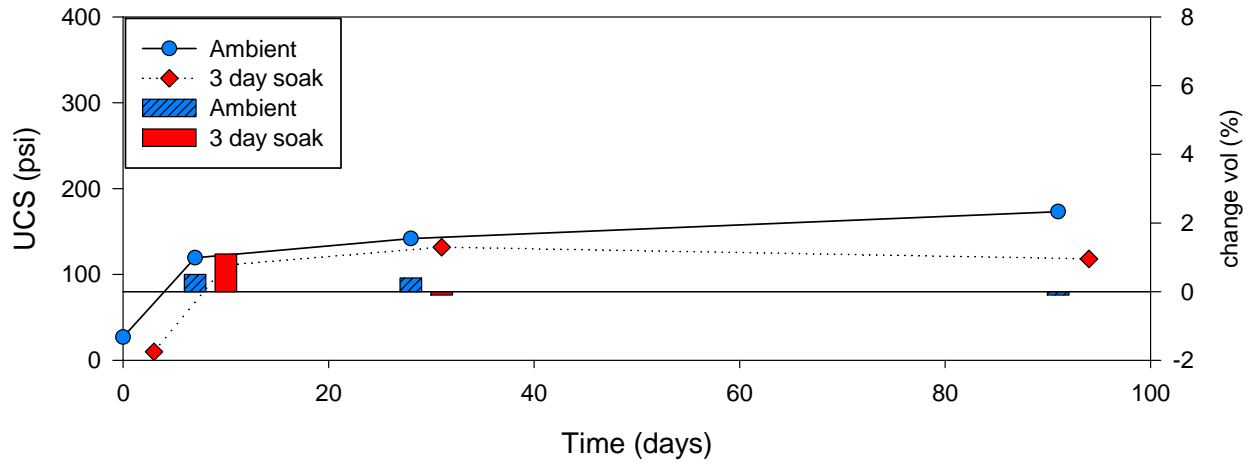
### S Soil Cement 4% with Fly Ash C 4%



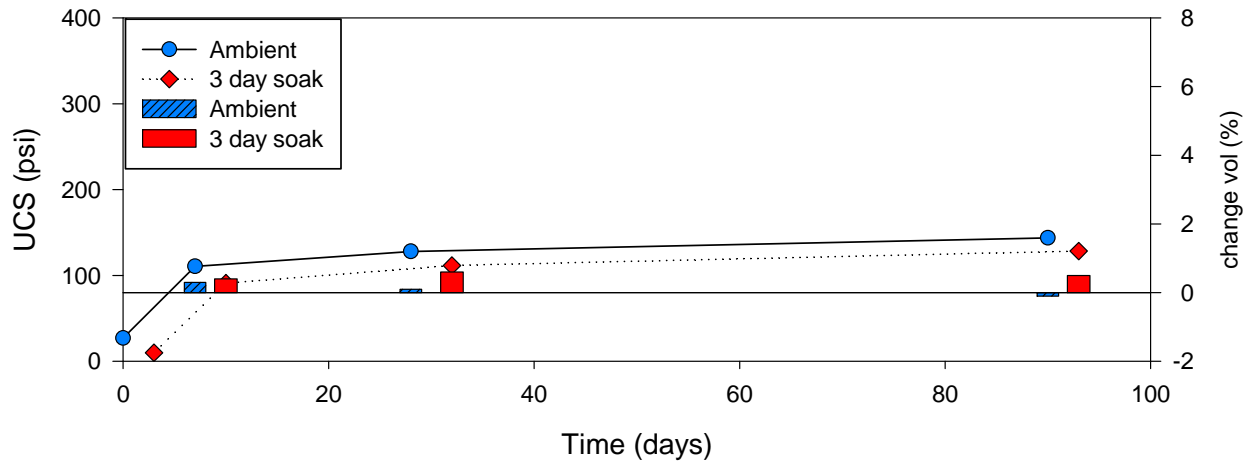
### S Soil Cement 3% with Fly Ash C 3%



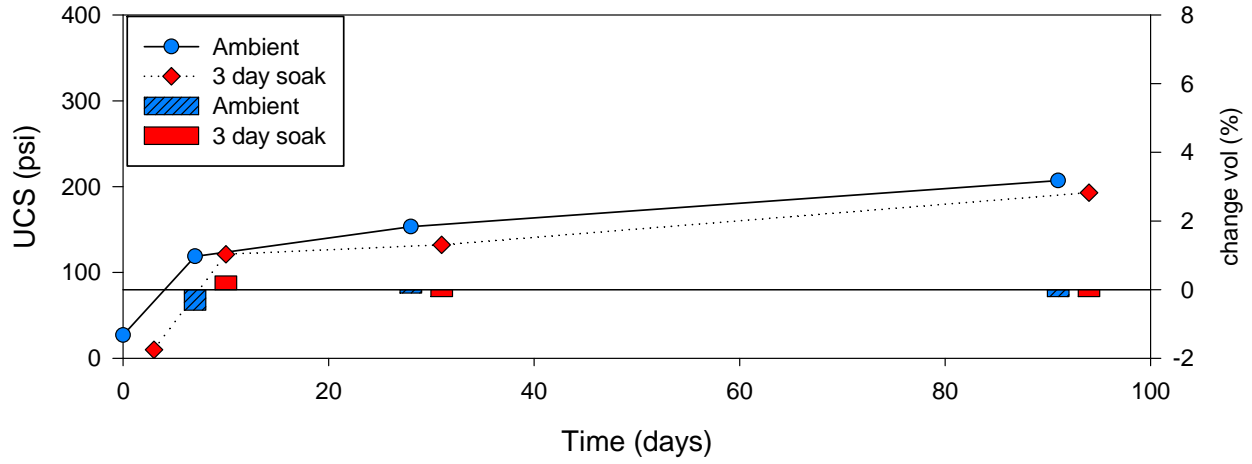
### S Soil Cement 8% with Fly Ash F 8%



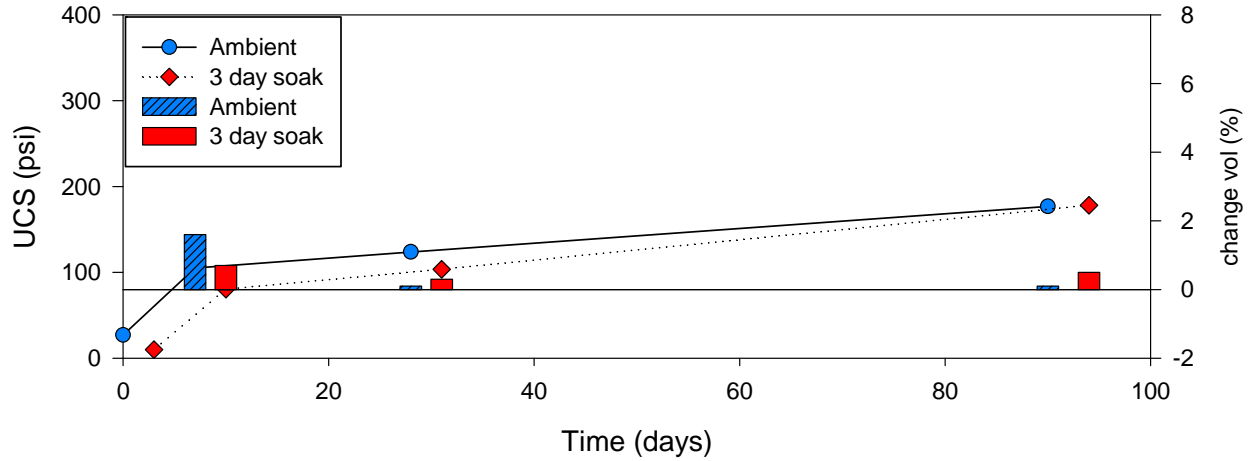
### S Soil Cement 4% with Fly Ash F 4%



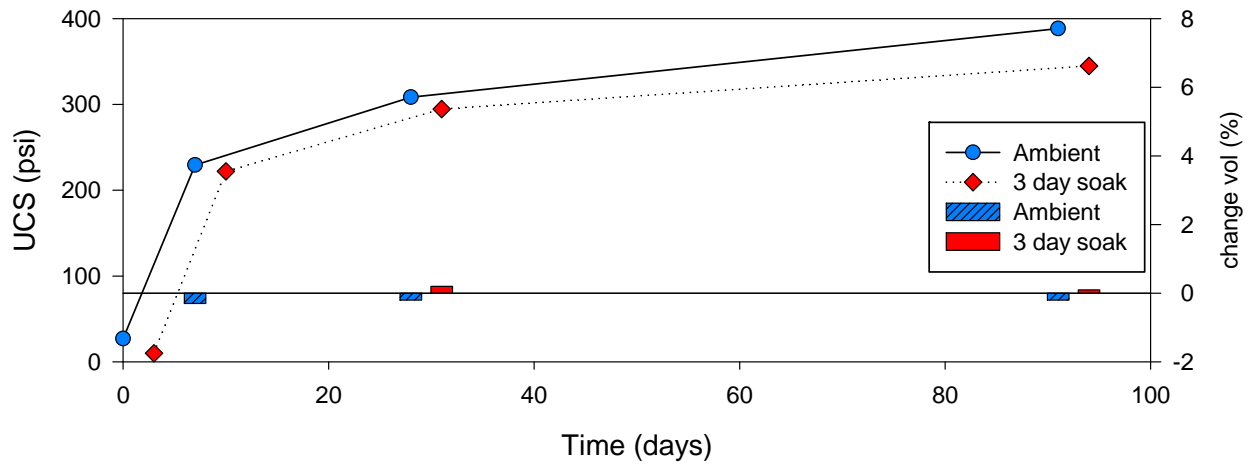
### S Soil Cement 8% with Slag 8%



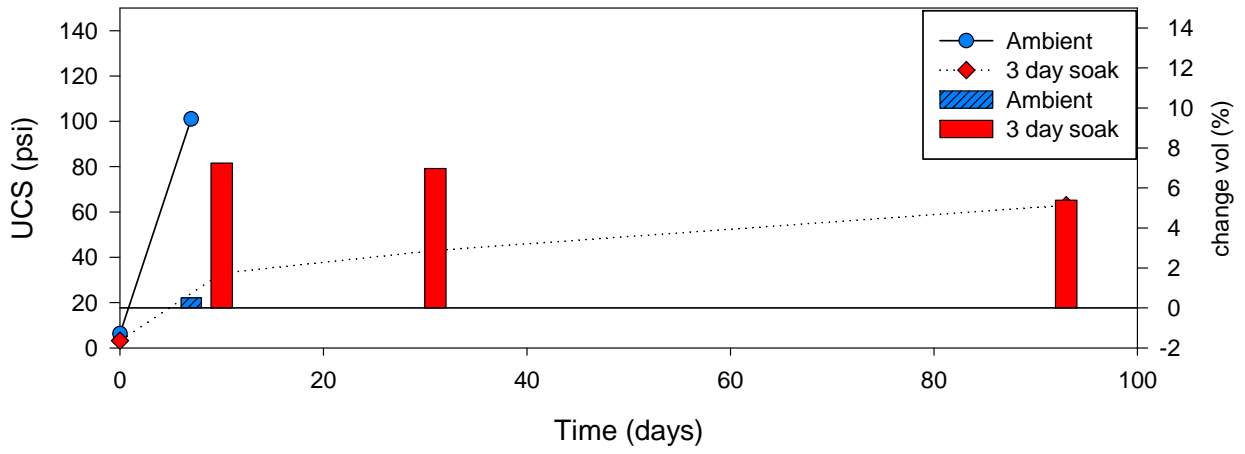
### S Soil Cement 4% with Slag 4%



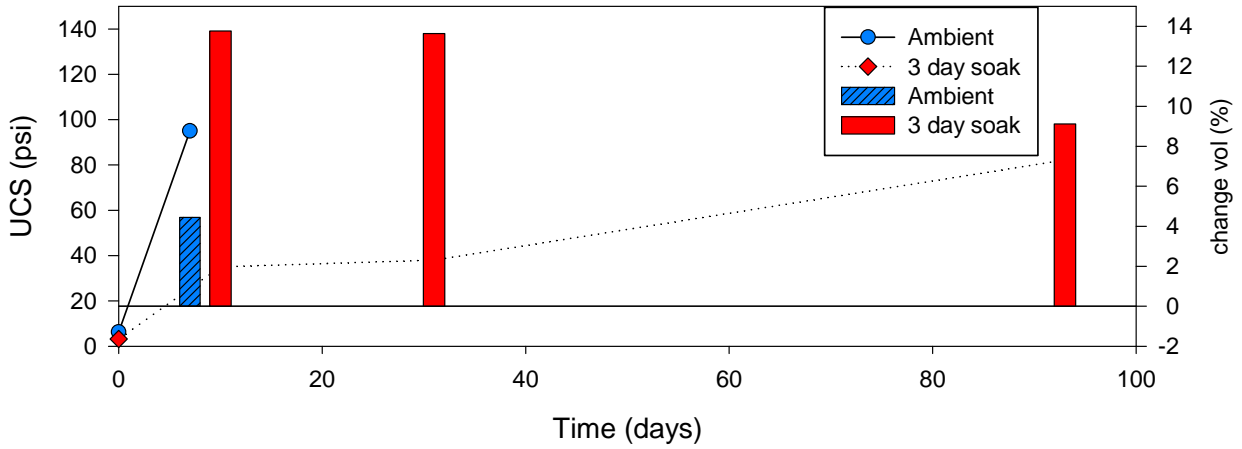
### S Soil Cement 8% with Lime 4%



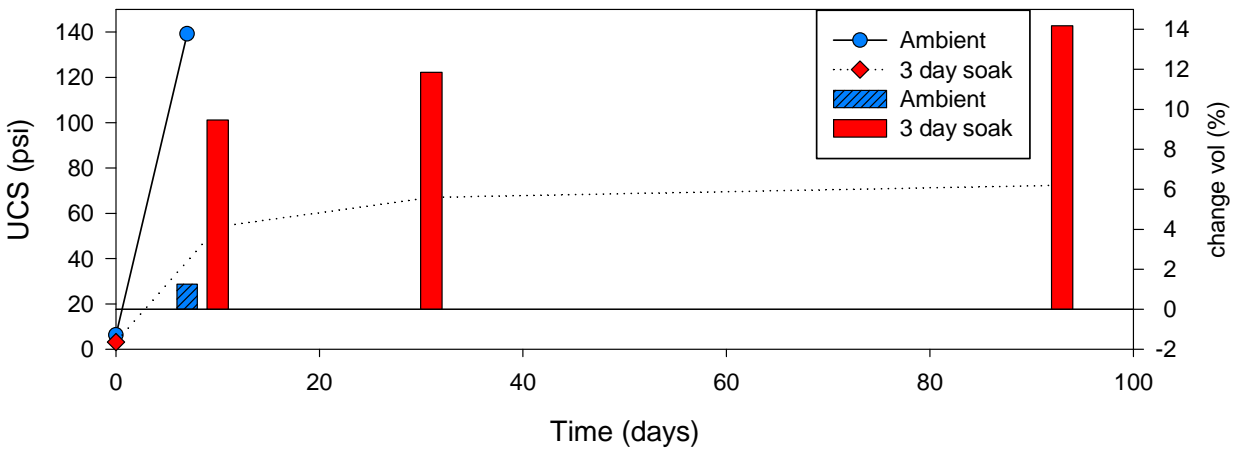
**? Soil Type II Cement 10%**



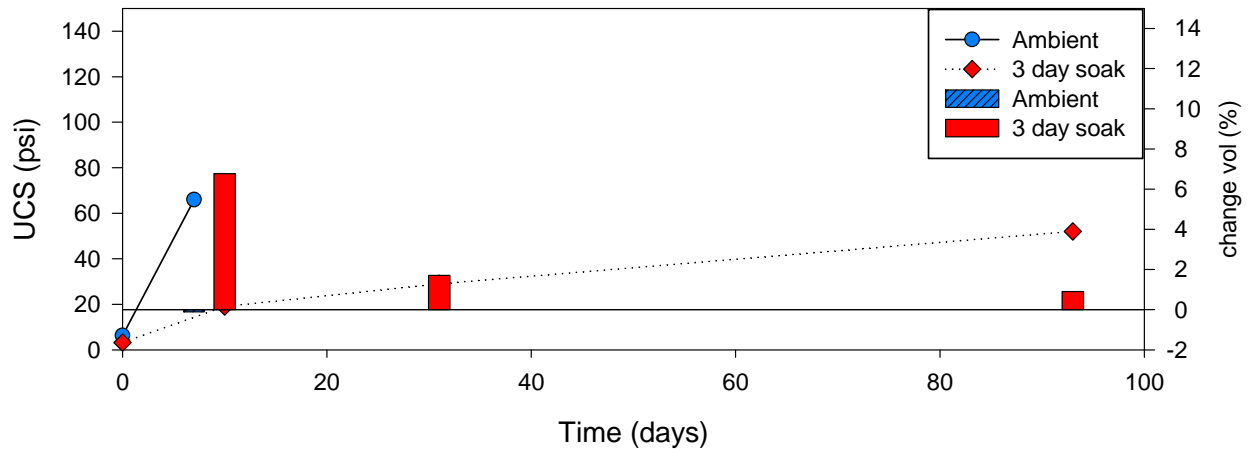
**? Soil Lime 5%**



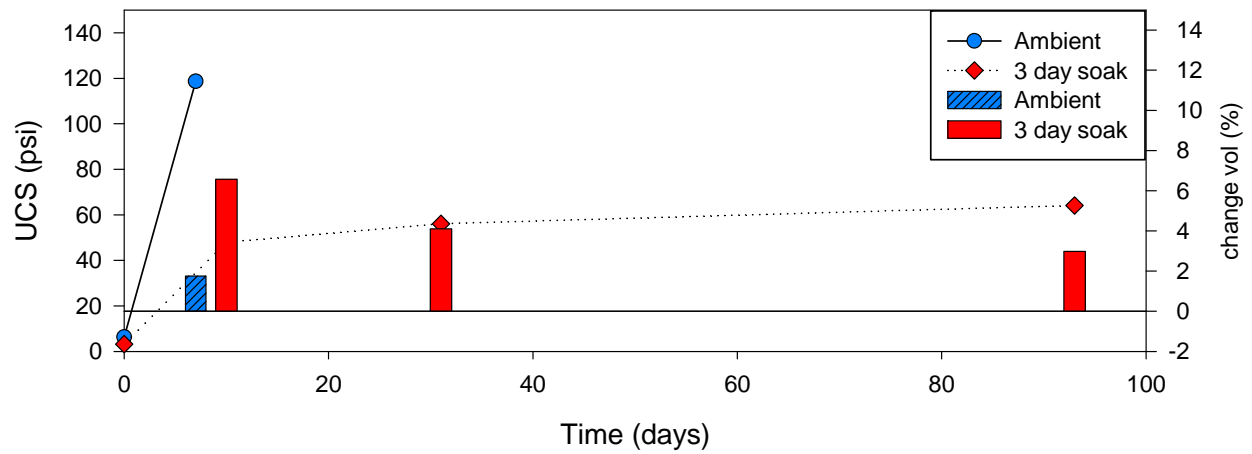
**? Soil Cement 8% with Lime 4%**



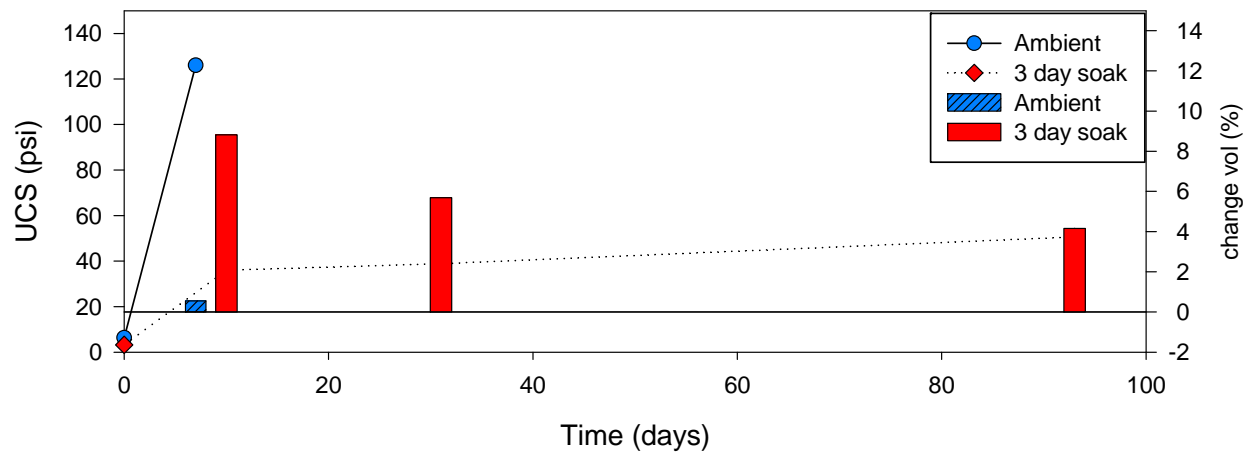
### ? Soil Class C Fly Ash 15%



### ? Soil Cement 8% with Slag 8%



### ? Soil Cement 8% with Fly Ash C 8%



### ? Soil Cement 8% with Fly Ash F 8%

